## Baseline and Future Year Mobility Conditions

North Carolina Department of Transportation
Strategic Transportation Corridor Vision Plans
Corridor X: Jacksonville to Greenville (U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258)
U.S. 17 in Onslow County to U.S. 64E in Edgecombe County

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## 1. Overview and Project Background

This memorandum presents base and future year mobility analyses for Corridor X (U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258) of the North Carolina Strategic Transportation Corridors (STC).

### 1.1. Overview of Strategic Transportation Corridors

In 2015, the North Carolina Department of Transportation (NCDOT) identified a network of key multimodal transportation corridors called Strategic Transportation Corridors (STC) to support smart planning, help set longterm investment decisions, and ensure that North Carolina's economic prosperity goals are achieved. The STCs are intended to promote transportation system connectivity, provide high levels of mobility, and improve access to important state and regional activity centers. A key element in the advancement of the STCs is the development of corridor master plan visions.

The purpose of the master plan visions is to:

- identify high-level corridor mobility visions and associated improvement strategies,
- guide improvements and development in a manner that defines a long-term vision and performance level for the corridors, and
- help protect the corridor's key functions as defined in the corridor profiles.


### 1.2. Corridor Description

U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 is approximately 90 miles in length and spans from Jacksonville to Greenville. U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 is primarily used to transfer freight from Jacksonville to Greenville. The corridor provides rural connection to economic development centers in Jacksonville, Kinston, and Greenville, including Camp Lejeune, Global TransPark, and East Carolina University. The principal expectation of the corridor is to provide safe, reliable mobility to these activity centers.

## 2. Highway Mobility

Highway mobility was analyzed for U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 for existing conditions and future scenarios based on the relationship of travel speed, congestion, and travel time. Existing conditions data was based on NCDOT traffic count data, GIS data, and third-party data (Google Maps satellite and travel time data). Future scenario analysis was based on the N.C. Statewide Travel Demand Model (NCSTM), Metropolitan Planning Organization (MPO) Travel Demand Models, the Statewide Transportation Improvement Program (STIP), and transportation plans from communities along the corridor.

### 2.1. Existing Conditions Analysis

Existing conditions analysis was completed using 2018 NCDOT Annual Average Daily Traffic (AADT) Segment Data, 2019 NCDOT Route Characteristics Data, the NCSTM, and third-party data (Google Maps). The 2015 NCSTM was used as the base year for most existing conditions analyses in this report. For some analyses, 2018 was used as the base year when more recent data was available. This section presents the process of identifying corridor segments and mobility measures.

### 2.1.1. Definitions of Segments

For analysis of the project corridor, the corridor was divided into "mobility segments." These segments represent sections that are generally homogenous and/or represent a uniform cross-section of roadway. The process of identifying segments included the review of the following attributes along the corridor:

- Major changes in roadway characteristics (cross-section, facility type, lanes)
- NCDOT Division Boundaries
- Interstate Crossings
- MPO Model boundaries
- Urban/rural transition

Segment breaks were not created for every occurrence of these characteristics: for example, small segments were avoided unless it was justified based on the uniqueness of the roadway attributes in that section. Although speed limits were a consideration, other factors were considered more heavily due to the frequency of speed limit changes.

A total of seven segments were identified for U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258, as shown in Table 1. These segments varied in length from 4 miles to 25 miles. Analysis was completed for these segments based on AADT information, NCDOT systems level planning capacities, NCSTM analysis, and MPO model analysis. The U.S.258/N.C. 11/U.S. 13 alignment is proposed to change based on the list of planned projects. The base year and future year segments are shown on Figure 1 and Figure 2.


Figure 1. Base Year (2015) Corridor Segments


Figure 2. Future Year (2040) Corridor Segments
The 2018 AADT is based on NCDOT AADT segment data, which contains different segments than the mobility segments defined for U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258. AADT for the mobility segments was calculated as a weighted average of the 2018 NCDOT AADT data within each segment. The 2018 AADT ranges and average AADT are presented in Table 1.

Table 1. U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Mobility Segments

| Segment | From | To | Length <br> (miles) | Division | 2018 AADT* <br> Range | Average 2018 <br> AADT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 101 | U.S. 64/U.S. 13 <br> Interchange | U.S. 264 | 12 | 2 | $650-22,000$ | 12,200 |
| 102 | U.S. 264 | N.C. 11/ N.C. 11 Bypass <br> Junction | 15 | 2 | $2,100-37,000$ | 23,700 |
| 103 | N.C. 11/ N.C. 11 Bypass <br> Junction | C.F. Harvey Parkway | 12 | 2 | $800-21,000$ | 15,800 |
| 104 | C.F. Harvey Parkway | U.S. 258/ N.C. 11 | 7 | 2 | $8,100-17,000$ | 13,300 |
| 105 | U.S. 258/N.C. 11 | N.C. 24 | 26 | 2 and 3 | $5,000-9,500$ | 6,800 |
| 106 | N.C. 24 | N.C. 53 | 14 | 3 | $11,500-44,500$ | 23,300 |
| 107 | N.C. 53 | U.S. 17/Lejeune Blvd <br> Interchange | 4 | 3 | $4,600-34,000$ | 21,900 |

[^0]
### 2.1.2. Segment Capacity and Travel Time

Typical planning-level highway capacity was developed for each segment along the corridor using the predominant cross-section representative of each segment. Capacities are based on NCDOT Transportation Planning Division's "Level of Service (LOS) D Standards for Systems Level Planning" (updated October 14, 2011) as shown in Appendix A. Segment facility type, typical number of lanes, area type, percent trucks, terrain, and travel speed were used to identify the daily planning-level capacity for comparison against traffic volumes. Segment capacities are shown in Table 2.

Travel times were calculated based on a weighted average of posted speeds for each segment (by length), existing Volume-to-Capacity (V/C) ratios, and a volume-delay curve similar to what is used in the NCSTM. Table 2 presents the travel time to traverse each segment based on this calculation. As a point of comparison, Google Maps travel times are provided for each segment to provide "observed" ranges based on third party data.

Table 2. Segment Capacity and Travel Times

| Segment | Facility Type | Typical Speed (mph) | Lanes | Median Type | Area Type | Planning Capacity | Travel Time |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  | Google Maps* | $\begin{aligned} & 2018 \\ & \text { Est.** } \end{aligned}$ |
| 101 | Boulevard | 55 | 4 | Divided | Rural | 49,000 | 10-13 | 13 |
| 102 | Boulevard | 45 | 4 | Divided | Suburban | 43,900 | 20-40 | 20 |
| 103 | Boulevard | 55 | 4 | Divided | Suburban | 43,900 | 9-11 | 13 |
| 104 | Major Thoroughfare | 50 | 4 | Divided | Suburban | 17,200 | 10-16 | 8 |
| 105 | Major Thoroughfare | 50 | 2 | Undivided | Rural | 16,400 | 28-35 | 31 |
| 106 | Major Thoroughfare | 55 | 4 | TWLTL | Rural | 35,700 | 15-20 | 15 |
| 107 | Major Thoroughfare | 45 | 6 | TWLTL | Suburban | 34,500 | 5-10 | 5 |

*Google Maps travel times captured during off-peak travel times in March 2020, prior to the COVID-19 Pandemic**2018 Estimated travel times calculated based on a weighted average of posted speeds for each segment, existing volume-to-capacity ratios, and a volume-delay curve

### 2.2. Future Scenario Analysis

Future scenario analysis was completed using growth rates developed for the corridor based on historical count data, the NCSTM, and relevant regional MPO models. Two future scenarios were analyzed, both which used the NCSTM Existing plus Committed ( $\mathrm{E}+\mathrm{C}$ ) scenario, which incorporates the fiscally constrained projects from the STIP. One scenario also includes local fiscally constrained and unconstrained projects in order to identify the local vision for the corridor:

- 2040 NCSTM E+C: Existing network plus committed (in the 2020-2029 STIP with either Right-ofWay/Construction funding) corridor projects
- 2040 NCSTM E+C + Metropolitan Transportation Plan (MTP)/Comprehensive Transportation Plan (CTP): NCSTM E+C (existing plus committed in STIP) plus fiscally constrained and unconstrained projects included in MTPs and CTPs
Typically, these projects are on the corridor itself; however, if the project is on a parallel facility and is of regional significance, it was included in the future scenario analysis. For each scenario, annual growth rates for each segment were prepared to project 2018 AADT to 2040. Using this information, future V/C ratio, travel time, average speed, vehicle-miles traveled (VMT), and vehicle-hours traveled (VHT) were calculated for each segment and the entire corridor. Population and employment growth data along the corridor are in Appendix G (based on the statewide travel demand model) and Appendix H (based on the regional travel demand models).


### 2.2.1. Committed and Fiscally Constrained Projects

For the $2040 \mathrm{E}+\mathrm{C}$ scenario, committed projects are those which are programmed in the 2020-2029 STIP. Table 3 shows projects included in the $2040 \mathrm{E}+\mathrm{C}$ scenarios for the U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 corridor.

Table 3. 2040 E+C Scenario Projects

| STIP ID | Segment | Counties | Roadway | Location/Description |
| :---: | :---: | :---: | :---: | :--- |
| R-2553C | 104 | Lenoir | U.S. 70 Bypass | Construct Kinston Bypass from NC 148 (Harvey Parkway) to <br> NC 58. |
| R-5703 | 104 | Lenoir | C.F. Harvey <br> Parkway | Construct N.C. 148 (C.F. Harvey Pkwy) on new location from <br> N.C. 58 to N.C. 11 |
| U-5716 | $106 / 107$ | Onslow | N.C. 24 | Convert at-grade intersection of N.C. 24 and U.S. 258 to an <br> interchange |
| U-5719 | 106 | Onslow | N.C. 24 | Realign Blue Creek Road/Ridge Road at U.S. 258/N.C. 24 to <br> form an at-grade intersection |

For the $2040 \mathrm{E}+\mathrm{C}+\mathrm{MTP} / \mathrm{CTP}$ scenario, the other fiscally constrained and unconstrained projects from the Greenville Urban Area MPO MTP, Kinston CTP, Edgecombe County CTP, Jones County CTP, Lenoir County CTP, and Pitt County CTP included in the project analysis are listed in Table 4.

Table 4. 2040 E+C + MTP/CTP Fiscally Constrained and Unconstrained Projects

| Plan | Segment | Counties | Roadway | Location/Description |
| :---: | :---: | :---: | :---: | :---: |
| CTP | 101 | Edgecombe | N.C. 11/U.S. 13 | Recommended for improvement from U.S. 13 to north of U.S. 64 |
| CTP | 101 | Edgecombe | U.S. 13 | Freeway Needs Improvement from U.S. 64 to Pitt County Line |
| CTP | 101 | Pitt | U.S. 13 | Freeway Needs Improvement from N.C. 30 to AllpineTaylor Road |
| MTP | 101 | Pitt | U.S. 13 | Recommended to upgrade to interstate standards, including shoulder work, access control, and interchange improvements from U.S. 13 from U.S. 264 to U.S. 64 |
| MTP | 102 | Pitt | U.S. 264/ Greenville Bypass | Widening from MPO boundary to US 264 |
| CTP | 103 | Lenoir | N.C. 11 | Upgrade existing facility to interstate standards from proposed Harvey Parkway Ext to Pitt County |
| CTP | 103 | Lenoir | N.C. 11 | Freeway Needs Improvement from Greenville SW Bypass to Hanrahan Road |
| MTP | 103 | Pitt | N.C. 11 | Interstate upgrade from Southwest Bypass to Pitt County Line |
| CTP | 103, 104 | Lenoir | N.C. 11 | Freeway Needs Improvement from Pitt County Line to N.C. 55 |
| CTP | 105 | Jones | U.S. 258 | Recommended to upgrade to expressway standards from Onslow County to Lenoir County |
| CTP | 105 | Lenoir | U.S. 258 | Other Major Thoroughfare Needs Improvement from Will Baker Road to Jones County Line |
| CTP | 105 | Lenoir | U.S. 258 | Recommended to be widened to a four-lane, mediandivided facility with control of access from proposed U.S. 258 and U.S. 70 interchange to Kinston planning boundary |
| CTP | 105 | Lenoir | U.S. 258 | Recommended to increase capacity from U.S. 70 Bypass to the southern planning boundary |
| CTP | 105 | Lenoir | U.S. 258 | Recommended to add an alternating passing lane, improve intersection visibility, add rumble stripes, and add more reflective thermoplastic paint from Jones County to Tyree Road (S.R. 1341) |
| CTP | 105 | Lenoir | U.S. 258 | Widen to a four-lane boulevard from Tyree Road to the proposed Kinston Bypass |

### 2.2.2. Existing and Future Cross-Sections

With the buildout of the $2040 \mathrm{E}+\mathrm{C}$ and $2040 \mathrm{E}+\mathrm{C}+$ MTP/CTP scenarios, the characteristics of each segment along the corridor change over time, typically resulting in higher throughput capabilities and increased travel speeds. Table 5 summarizes the facility type, lanes and typical posted speed for 2018, $2040 \mathrm{E}+\mathrm{C}$ and $2040 \mathrm{E}+\mathrm{C}$ + MTP/CTP scenarios. Shaded grey fields indicate a change from existing ("2018 Conditions") to the 2040 scenarios.

Table 5. Volume-to-Capacity Ratios by Scenario

| Segment 2018 Conditions | 2040 NCSTM E+C |  |  | 2040 NCSTM E+C + MTP/CTP |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Facility Type | Typical <br> Posted <br> Speed | Lanes | Facility Type | Typical <br> Posted <br> Speed | Lanes | Facility Type | Typical <br> Posted <br> Speed | Lanes |
| 101 | Boulevard | 55 | 4 | Boulevard | 55 | 4 | Freeway | 70 | 4 |
| 102 | Boulevard | 45 | 4 | Freeway | 70 | 4 | Freeway | 70 | 4 |
| 103 | Boulevard | 55 | 4 | Boulevard | 55 | 4 | Freeway | 60 | 4 |
| 104 | Major <br> Thoroughfare | 50 | 4 | Expressway | 60 | 4 | Expressway | 60 | 4 |
| 105 | Major <br> Thoroughfare | 50 | 2 | Major <br> Thoroughfare | 50 | 2 | Major <br> Thoroughfare | 55 | 3 |
| 106 | Major <br> Thoroughfare | 55 | 4 | Major <br> Thoroughfare | 55 | 4 | Major <br> Thoroughfare | 55 | 4 |
| 107 | Major <br> Thoroughfare | 45 | 6 | Major <br> Thoroughfare | 45 | 6 | Major <br> Thoroughfare | 45 | 6 |

Note: Shaded grey fields indicate a change from 2018 Conditions to 2040 scenarios

### 2.2.3. Travel Demand Model Analysis

Travel Demand Model analysis was completed using the NCSTM. The most recent NCSTM uses a base year of 2015 and a future year of 2040. Data from this model was used to calculate growth rates. Table 6 presents NCSTM model output related to volumes and speeds from the 2015 and $2040 \mathrm{E}+\mathrm{C}$ network.

Table 6. 2015/2040 NCSTM E+C Comparison

| Segment | 2015 NCSTM |  |  |  | 2040 NCSTM E+C |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Ave. AADT* | Daily VMT** | Daily VHT*** | Ave. <br> Speed <br> $(\mathbf{m p h})$ | Ave. AADT | Daily VMT | Daily VHT | Ave. <br> Speed <br> $(\mathbf{m p h})$ |
| 101 | 6,000 | 136,100 | 2,500 | 55 | 8,000 | 195,900 | 1,800 | 55 |
| 102 | 6,000 | 165,800 | 4,100 | 42 | 10,000 | 393,000 | 2,800 | 70 |
| 103 | 6,000 | 319,700 | 20,100 | 54 | 9,000 | 207,800 | 3,800 | 55 |
| 104 | 9,000 | 204,800 | 12,700 | 49 | 4,000 | 95,500 | 1,800 | 52 |
| 105 | 7,000 | 138,600 | 24,800 | 51 | 9,000 | 333,000 | 6,600 | 51 |
| 106 | 17,000 | 213,500 | 4,000 | 53 | 21,000 | 294,200 | 5,500 | 55 |
| 107 | 12,000 | 64,100 | 1,700 | 38 | 17,000 | 97,000 | 2,000 | 43 |
| Total | 9,000 | $1,242,600$ | 69,900 | 49 | 11,143 | $1,616,400$ | 24,300 | 54 |

*AADT = Annual Average Daily Traffic; **VMT = Vehicle-Miles Traveled; ***VHT = Vehicle-Hours Traveled
MPO models were also used as part of the Travel Demand Model analysis. For the MPO models, an E+C scenario was not evaluated; rather, the adopted MTPs were utilized for future year analysis. Information from these models was used to support development of growth rates to apply to each segment. Table 7 shows a
comparison of the MPO data. When comparing growth data from the NCSTM and MPO models, it should be noted that corridor segments may be represented in multiple local travel demand model models or only partially represented.

Table 7. Base Year and Future Year Scenario, MPO Model Output

|  |  |  |  | Base Year Data |  |  |  | Future Year Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment | Travel Demand Model | Base Year | Future Year | Ave. AADT* | Daily VMT** | Daily VHT*** | Ave. Speed (mph) | Ave. AADT | Daily VMT | Daily VHT | Ave. Speed (mph) |
| 101 | Greenville | 2016 | 2045 | 8,200 | 174,500 | 700 | 57 | 10,100 | 216,900 | 900 | 57 |
| 102 | Greenville | 2016 | 2045 | 14,100 | 165,800 | 1,000 | 38 | 13,900 | 166,400 | 1,000 | 38 |
| 103 | Greenville/ Kinston | 2015 | 2045 | 10,000 | 236,100 | 1,900 | 49 | 9,500 | 234,700 | 1,800 | 49 |
| 104 | Kinston | 2015 | 2045 | 10,000 | 42,800 | 1,000 | 38 | 10,100 | 38,200 | 900 | 38 |
| 105 | Greenville/ Kinston | 2010 | 2045 | 6,700 | 46,600 | 1,200 | 43 | 7,200 | 54,700 | 1,400 | 40 |
| 106 | Jacksonville | 2010 | 2040 | 17,500 | 251,900 | 5,500 | 51 | 25,100 | 373,200 | 8,700 | 43 |
| 107 | Jacksonville | 2010 | 2040 | 16,200 | 83,900 | 2,300 | 42 | 21,900 | 113,400 | 3,500 | 34 |

*AADT = Annual Average Daily Traffic; **VMT = Vehicle-Miles Traveled; ***VHT = Vehicle-Hours Traveled

### 2.2.4. Projected Growth Rates

Projected growth rates were developed based on AADT data from the NCSTM and MPO models by corridor segment. Table 8 shows the projected growth rate for each corridor segment.

Table 8. Projected Growth Rates by Segment

|  | NCSTM/MTP/CTP |  | MPO |  | STC Growth Rate |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment | Annual Growth Rate, <br> $2015-2040$ E+C | Annual Growth Rate, <br> $2015-2040$ E+C + MTP/CTP | Annual <br> Growth Rate | E+C + MPO | E+C MTP/CTP <br> + MPO |  |
| 101 | $1.2 \%$ | $2.5 \%$ | $0.7 \%$ | $0.9 \%$ | $1.7 \%$ |  |
| 102 | $2.1 \%$ | $2.1 \%$ | $0.0 \%$ | $1.0 \%$ | $1.5 \%$ |  |
| 103 | $1.6 \%$ | $1.6 \%$ | $-0.2 \%$ | $0.7 \%$ | $1.2 \%$ |  |
| 104 | $-3.2 \%$ | $0.0 \%$ | $0.0 \%$ | $-1.8 \%$ | $-0.8 \%$ |  |
| 105 | $1.0 \%$ | $1.8 \%$ | $0.2 \%$ | $0.6 \%$ | $1.2 \%$ |  |
| 106 | $0.8 \%$ | $1.0 \%$ | $1.2 \%$ | $1.0 \%$ | $1.0 \%$ |  |
| 107 | $1.4 \%$ | $1.4 \%$ | $1.0 \%$ | $1.2 \%$ | $1.3 \%$ |  |

### 2.3. Mobility Measures

While there are many mobility measures that can be considered for each corridor based on quantitative and qualitative data, this mobility analysis is based on the relationship of travel speed, congestion, and travel time. For each scenario, a projected volume was compared against available capacity to estimate the travel time (i.e., V/C ratio). VMT, VHT, and average speed are also presented for each scenario.

### 2.3.1. Volume-to-Capacity Ratio

The V/C ratio is a representation of a daily planning-level capacity versus an average daily traffic volume. It is not a measure of peak traffic or congestion, but rather an overall measure of the how well the roadway will function over the course of a day. According to the "Level of Service D Standards for Systems Level Planning," typical
capacities shown are the points at which traffic transitions from LOS D to LOS E; therefore, segments with a V/C ratio exceeding 1.0 are considered greater than LOS D (i.e., LOS E or $F$ ) in this analysis. Table 9 presents V/C ratios by scenario. Shaded grey fields indicate a change from existing ("2015 NCSTM") to the 2040 scenarios

Table 9. Volume-to-Capacity Ratios by Scenario

| Segment | 2015 NCSTM |  |  |  | 2040 NCSTM E+C |  |  | 2040 NCSTM E+C + MTP/CTP |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Ave. Vol. Capacity | Ave. V/C* | Ave. Vol. | Capacity | Ave. V/C | Ave. Vol. | Capacity | Ave. V/C |  |
| 101 | 5,980 | 49,000 | 0.12 | 8,490 | 49,000 | 0.17 | 11,500 | 64,700 | 0.18 |
| 102 | 6,700 | 39,700 | 0.17 | 10,300 | 62,400 | 0.17 | 10,400 | 62,400 | 0.17 |
| 103 | 6,800 | 43,900 | 0.15 | 9,150 | 43,900 | 0.21 | 9,100 | 65,400 | 0.14 |
| 104 | 9,750 | 17,200 | 0.57 | 3,560 | 57,100 | 0.06 | 9,500 | 57,100 | 0.17 |
| 105 | 6,930 | 16,400 | 0.42 | 9,590 | 16,400 | 0.58 | 12,200 | 17,800 | 0.69 |
| 106 | 17,740 | 35,700 | 0.50 | 22,800 | 35,700 | 0.64 | 23,000 | 35,700 | 0.64 |
| 107 | 12,330 | 34,500 | 0.36 | 18,440 | 34,500 | 0.53 | 18,400 | 34,500 | 0.53 |

*V/C = Volume-to-Capacity ratio
Note: Shaded grey fields indicate an increase from 2015

### 2.3.2. Average Travel Time and Speed

Average travel time and speed are measures of the relationship between the V/C ratio of a segment and its typical travel speed. Volume-delay curves by facility type were used to estimate travel time and speed in the 2018 conditions. These volume-delay curves, based on adjusted NCSTM volume-delay function curves, represent the typical "congested" speed on a daily planning level. Table 10 shows average travel time and speeds by scenario. Shaded grey fields indicate a change from existing ("2018 Conditions") to the 2040 scenarios.

Table 10. Average Travel Speed and Travel Time by Scenario

| Segment | 2018 Conditions |  |  | 2040 NCSTM E+C |  |  | 2040 NCSTM E+C + MTP/CTP |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Typical <br> Posted <br> Speed <br> $(\mathrm{mph})$ | Ave. <br> Travel <br> Speed <br> $(\mathrm{mph})$ | Ave. <br> Travel <br> Time <br> $(\mathrm{min})$ | Typical <br> Posted <br> Speed <br> $(\mathrm{mph})$ | Ave. <br> Travel <br> Speed <br> $(\mathrm{mph})$ | Ave. <br> Travel <br> Time <br> $(\mathrm{min})$ | Typical <br> Posted <br> Speed <br> $(\mathrm{mph})$ | Ave. <br> Travel <br> Speed <br> $(\mathrm{mph})$ | Ave. <br> Travel <br> Time <br> $(\mathrm{min})$ |
|  | 55 | 53 | 13 | 55 | 55 | 14 | 70 | 66 | 11 |
| 102 | 45 | 41 | 20 | 70 | 70 | 15 | 70 | 69 | 15 |
| 103 | 55 | 51 | 13 | 55 | 55 | 13 | 60 | 56 | 13 |
| 104 | 50 | 41 | 8 | 60 | 52 | 18 | 60 | 57 | 8 |
| 105 | 50 | 46 | 31 | 50 | 51 | 31 | 55 | 55 | 30 |
| 106 | 55 | 48 | 15 | 55 | 55 | 16 | 55 | 55 | 16 |
| 107 | 45 | 40 | 5 | 45 | 43 | 14 | 45 | 43 | 6 |
| Total Travel Time (min) |  |  |  |  |  |  |  |  | $\mathbf{1 0 7}$ |

[^1]
### 2.3.3. Vehicle-Miles Traveled and Vehicle-Hours Traveled

VMT and VHT represent overall demand on each segment for each scenario, shown on Table 11.
Table 11. VMT and VHT Scenario

| Segment | 2015 NCSTM |  | 2040 NCSTM E+C |  | 2040 NCSTM E+C + MTP/CTP |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | VMT | VHT | VMT | VHT | VMT | VHT |
| 101 | 136,100 | 2,500 | 195,900 | 1,800 | 267,600 | 2,000 |
| 102 | 165,800 | 4,100 | 393,000 | 2,800 | 395,900 | 2,800 |
| 103 | 319,700 | 20,100 | 207,800 | 3,800 | 208,900 | 1,900 |
| 104 | 204,800 | 12,700 | 95,500 | 1,800 | 125,300 | 1,500 |
| 105 | 138,600 | 24,800 | 333,000 | 6,600 | 358,500 | 6,700 |
| 106 | 213,500 | 4,000 | 294,200 | 5,500 | 296,900 | 5,500 |
| 107 | 64,100 | 1,700 | 97,000 | 2,000 | 96,900 | 2,000 |
| Total | $\mathbf{1 , 2 4 2 , 6 0 0}$ | $\mathbf{6 9 , 9 0 0}$ | $\mathbf{1 , 6 1 6 , 4 0 0}$ | $\mathbf{2 4 , 3 0 0}$ | $\mathbf{1 , 7 4 9 , 9 0 0}$ | $\mathbf{2 2 , 4 0 0}$ |

*VMT = Vehicle-Miles Traveled; **VHT = Vehicle-Hours Traveled

### 2.3.4. Highway Mobility Summary

Table 12 presents a summary of highway mobility measures for 2015 NCSTM, 2040 NCSTM E+C, and 2040 NCSTM E+C + MTP/CTP. The table shows that in both 2040 scenarios, the corridor serves more travelers at higher speeds with less delay. Figure 3 presents the key highway mobility measures graphically.

Table 12. Highway Mobility Summary

| Measure | 2015 NCSTM | 2040 NCSTM E+C | 2040 NCSTM E+C + MTP |
| :--- | :---: | :---: | :---: |
| Length (Miles) | 90 | 103 | 103 |
| Average Travel Time (Hours) | 1.83 | 1.70 | 1.63 |
| Vehicle-Miles Traveled | $1,242,600$ | $1,616,400$ | $1,749,900$ |
| Vehicle-Hours Traveled | 69,900 | 24,300 | 22,400 |
| Annual Average Daily Volume | 9,461 | 11,761 | 13,443 |
| Average Speed | 49 | 54 | 57 |

## Highway Mobility Benefits

## STC Corridor X

U.S. 17 in Onslow County to U.S. 64E in Edgecombe County


VTM (MILLIONS)


Figure 3. Highway Mobility Summary

### 2.4. Truck Percentage

The percent of trucks on the corridor was reviewed using AADT GIS data from NCDOT, which is collected for routes on the National Highway System and the North Carolina Truck Network. Truck percentage data and maps are in Appendix E.

### 2.5. Electric Charging Stations

Electric charging stations within a 5 -mile and 10-mile buffer of the corridor are illustrated on figures in Appendix F .

## 3. Freight Mobility

The U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor runs from U.S. 17 in Onslow County near Jacksonville to U.S. 64 East in Edgecombe county near Greenville along segments of U.S. 258, N.C. 11, and U.S. 13 as shown in Figure 4. Freight mobility into, out of, and within the U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor counties was analyzed using freight flow data downloaded from the North Carolina Freight Flow tool. The freight flow data is presented as volume (tonnage) and value (dollars). It is based on the Federal Highway Administration's (FHWA) Freight Analysis Framework Version 4.1 (FAF4.1) with county-level disaggregation processed by Cambridge Systematics for 2012 and 2015, and it was forecasted to 2045 using FHWA's FAF4.1 origin-destination and commodity growth rates for rail flows. ${ }^{1}$

[^2]

Figure 4. U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor
Freight flow estimates for the U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor include county totals for five counties (Edgecombe, Pitt, Lenoir, Jones, and Onslow) within eastern North Carolina. Results are presented for 12 different commodity groups and associated trade partners. Results by trade partners are presented regionally for the United States, at the county level for trade between the corridor and the rest of North Carolina, and at the FAF regional level for all other trade which includes states, large metropolitan areas, the remainder of states with large metropolitan area(s), and international regions for foreign freight flows.

### 3.1. Flow Type Totals

Freight flows to, from, and within the U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor counties (including domestic trade and the domestic leg of foreign trade) totaled an estimated 21.7 million tons worth $\$ 27.4$ billion in 2015, as shown in Figure 5. Inbound flows represented a higher percentage of volume compared to value, while outbound flows represented a lower percentage of volume compared with value. Internal flows to the corridor made up only 1 percent of the volume and less than 1 percent of the total value. Flows were forecasted to increase to 31.1 million tons worth $\$ 47.5$ billion in 2045 (an increase of about 43 and 73 percent, respectively). The differences in growth of volume and value reflect anticipated changes in type of businesses.

Total Tons 2015: 21.7 million


Total Value 2015: $\$ 27.4$ billion


Figure 5. Freight Flow Totals, 2015

### 3.2. Modal Splits

Trucking dominates the market, moving approximately 92 percent of the corridor's freight and accounting for almost 90 percent of the total value, as shown in Figure 6 and

Figure 7. Pipeline approximately 5 percent of volume and value in 2015 , while carload rail carried about 3 percent of the total volume and 1 percent of the total value, and air cargo carried less than 1 percent of the total volume but about 3 percent of the total value. Water cargo and other modes of freight movement represented minimal volume and value, at less than 1 percent, respectively. Modal share forecasts for 2045 show little change in terms of volume, but indicate a small decrease in air and truck freight by about two and 3 percent share of the total value, respectively. Conversely, water freight's share of the total value is projected to increase by almost 4 percent of the total value.

2015 Tons: 21.7 million


Air - Other - Pipeline - Rail (Carload) - Truck = Water

2015 Value: $\$ 27.4$ billion


Figure 7. Modal Freight Flows by Value, 2015

### 3.3. Commodity Comparison, 2015 and 2045

Aggregates, with about 4.0 million tons, accounted for the largest volume of commodities moving into, out of, and within the corridor, shown in Figure 8, of which approximately 64 percent was imported to the region. Agriculture and Fish; Nonmetallic Mineral and Base Metal Products; Chemicals, Pharmaceuticals, Plastics, and Rubber; Raw and Finished Wood Products; and Energy Products each accounted for more than 2.0 million tons of freight volume in 2015. By 2045, flows of Machinery, Electric, and Precision Instrument and Chemicals,

Pharmaceuticals, Plastics, and Rubber are forecasted to increase by approximately 153 percent and 102 percent, respectively. This growth is forecasted to make Chemicals, Pharmaceuticals, Plastics and Rubber the largest traded commodity by volume in 2045. Other commodity groups with high growth forecasts include Waste (73 percent), Mixed Freight ( 64 percent), and Food, Alcohol, and Tobacco (61 percent). Energy Products are the only commodity with an anticipated decrease ( 20 percent) in volumes by 2045.


Figure 8. Commodity Volumes, 2015 and 2045

As shown in Figure 9, Mixed Freight, at more than $\$ 6.4$ billion, accounted for the largest share of the value of freight in 2015, which was closely followed by Chemicals, Pharmaceuticals, Plastics, and Rubber at $\$ 6.2$ billion. The forecasted growth by 2045 is 68 percent for Mixed Freight and 97 percent for Chemicals, Pharmaceuticals, Plastics, and Rubber. This makes Chemicals, Pharmaceuticals, Plastics, and Rubber the largest commodity by value of freight by 2045. Machinery, Electric, and Precision Instruments is forecasted to have the highest growth in value by 2045 ( 135 percent), but remains the third highest traded commodity by value in 2045 . Other commodities with high forecasted growth by 2045 include Waste ( 90 percent), Aggregates ( 62 percent), Food, Alcohol, and Tobacco ( 62 percent), Nonmetallic Mineral and Base Metal Products ( 59 percent), and Agriculture and Fish ( 57 percent). The only commodity with a forecasted decline in value by 2045 ( 24 percent) is Energy Products.


Figure 9. Commodity Values, 2015 and 2045

### 3.4. Top Trading Partners — by Volume and Value

The U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor counties traded the largest volume and value of goods within North Carolina when compared to all other U.S. trading regions identified in Figure 10. In 2015, this was estimated to be over 18.8 million tons valued at over $\$ 17.6$ billion with a forecasted growth to 26.5 million tons worth $\$ 28.9$ billion by 2045, as shown in Table 13. In 2015, trade within North Carolina represented more than 80 percent of the volume and 60 percent of the value of all trade and is forecasted to maintain that level in 2045. Outside of North Carolina, the largest trading partner by volume and value is within the Southeast region. The Southeast region represented just over 1.1 million tons of freight worth about $\$ 4.2$ million which represented about 40 percent of the trade volume and value outside of North Carolina.

Table 13. Top Regional Trading Partners

| Region | Tonnage |  | Value |  |
| :--- | :--- | :--- | :--- | :--- |
|  | $\mathbf{2 0 1 5}$ | $\mathbf{2 0 4 5}$ | $\mathbf{2 0 1 5}$ | $\mathbf{2 0 4 5}$ |
| Internal (North Carolina) | $18,833,145$ | $26,551,106$ | $\$ 17,567,694,320$ | $\$ 28,943,755,438$ |
| Great Lakes | 400,568 | 649,458 | $\$ 1,822,697,313$ | $\$ 3,222,110,419$ |
| Mideast | 857,869 | $1,385,963$ | $\$ 1,852,893,330$ | $\$ 3,659,941,574$ |
| New England/New York | 124,812 | 239,608 | $\$ 579,842,401$ | $\$ 1,326,535,162$ |
| Southeast ${ }^{*}$ | $1,165,525$ | $1,788,244$ | $\$ 4,231,480,999$ | $\$ 8,041,290,838$ |
| West of the Mississippi River | 282,830 | 470,340 | $\$ 1,354,963,856$ | $\$ 2,339,474,346$ |
| Total | $\mathbf{2 1 , 6 6 4 , 7 4 9}$ | $\mathbf{3 1 , 0 8 4 , 7 2 0}$ | $\$ 27,409,572, \mathbf{2 1 8}$ | $\$ 47,533,107,777$ |

*Freight internal to North Carolina was excluded from totals within the Southeast


Figure 10. Trading Regions

Figure 11 shows the top ten domestic trading partners with the U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor by volume by county within NC, metropolitan area, and "other" state FAF region outside of NC. "Other" state FAF regions refer to the remainder of a state trading region which does not include separately analyzed metropolitan areas. New Hanover County, N.C. and New Orleans, LA were the two largest trade partners for the U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor in 2015 by volume and represented a combined value of more than 1.3 million tons of freight. New Hanover County is also forecasted to grow to 1.2 million tons by 2045, which represents the second largest growth in volume ( 72 percent) within the top ten trade partners by 2045. Virginia Beach, VA, which had the largest growth in volume, is forecasted to grow 85 percent by 2045. This increase in trade with Virginia Beach, VA will bring it from the tenth largest trading partner by volume to the sixth largest in 2045.


Figure 11. Top Trading Partners by Volume, 2015 and $2045^{2}$

[^3]Atlanta, GA, far outranked the other top ten trade partners by value for the U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor in 2015, as shown in Figure 12. Atlanta represented more than $\$ 2.1$ billion of trade in 2015, of which approximately 82 percent was outbound freight. Atlanta freight movements are forecasted to grow by 78 percent in value by 2045 to more than $\$ 3.8$ billion. New Hanover County, N.C. and Other S.C. were the second and third largest trading partners by value in 2015 , with approximately $\$ 770$ million and over $\$ 690$ million in freight trade, respectively. The remaining top ten trade partners by value for the U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 Corridor ranged between $\$ 400$ million and $\$ 660$ million worth of trade in 2015. Savannah, GA, Charleston, SC, Virginia Beach, VA, New Hanover County, NC, Los Angeles, CA, and Chicago, IL are anticipated to more than double in value by 2045 with growths of 252 percent, 234 percent, 174 percent, 119, percent, 107 percent, and 104 percent, respectively. These large growths make Charleston, S.C. and Virginia Beach, VA the projected second and third largest trading partners by value in 2045, respectively. Other trade partners with high increases in value by 2045 are Columbus, OH and Philadelphia, PA with growth of 93 percent and 55 percent, respectively.


Figure 12. Top Trading Partners by Value, 2015 and $2045^{3}$

[^4]
### 3.5. Foreign Trade

Foreign trade freight flows of 908,000 tons only represented approximately 4 percent of the corridor's total flows in 2015 and is forecasted to increase in volume by 165 percent to become almost 8 percent of the total volume ( 31.1 million tons) by 2045. The $\$ 6.3$ billion worth of foreign trade in 2015 is forecasted to grow by 58 percent to $\$ 9.9$ billion by 2045 . Foreign trade flows account for 23 percent and 32 percent of total trade freight flows by value in 2015 and 2045, respectively.

As shown in Figure 13, tonnage of foreign trade in 2015 is dominated by water with 85 percent of freight being moved on the water and trucking and carload rail ranking second at 7 percent each. As shown in Figure 14, modal shares of foreign trade by value in 2015 are also dominated by water which accounts for 74 percent of the total, with truck freight ranking second at 16 percent.

The modal share by volume forecasted between 2015 and 2045 remains relatively the same with a small relative decrease in water freight ( 85 to 81 percent) and increases in truck and carload rail freight ( 7 to 9 percent each). However, the modal share by value is forecasted to increase for water ( 74 to 79 percent) and decrease in air freight ( 5 to 1 percent).


Figure 13. Foreign Trade Freight Flows by Mode and Volume, 2015

Chemicals, Pharmaceuticals, Plastics, and Rubber was the top foreign traded commodity group by volume in 2015 with more than 230,000 tons, which represented 26 percent of the total foreign trade volume, as shown in Figure 15. By 2045 it is forecasted to increase by 183 percent, which would be about 28 percent of the total foreign trade by volume and remain the top foreign traded commodity group by volume. Aggregates was also a high volume foreign traded commodity with about 200 thousand tons of freight traded in 2015, making it the second highest traded commodity in 2015 . However, Aggregates is forecasted to have the least growth ( 45 percent) by 2045 compared to all the other trade commodities, with the exception of Energy Products which is forecasted to decline by 80 percent. By 2045, all of the other commodities are forecasted to grow in volume by more than 120 percent, and by as much as 410 percent. As a result, Aggregates is projected to drop to the third most traded commodity in 2045 and Machinery, Electric, and Precision Instruments is projected to become the second most traded commodity.


Figure 15. Foreign Trade Commodity Volumes, 2015 and 2045

By value, Machinery, Electric, and Precisions Instruments accounted for 30 percent of the total value of foreign trade value with $\$ 1.0$ billion, as shown in Figure 16. By 2045, the same commodity group is forecasted to account for 36 percent of the total value of foreign trade with over $\$ 3.5$ billion ( 243 percent growth). Chemicals, Pharmaceuticals, Plastics, and Rubber and Mixed Freight were the second and third highest traded commodities by value in 2015 at $\$ 673$ and $\$ 610$ million, respectively. These two commodities are forecasted to grow in value by 164 percent to $\$ 1.8$ billion and 212 percent to $\$ 1.9$ billion, respectively, by 2045 . Similar to the projected growth in volume, all of the other trade commodities are anticipated to grow in value by more than 110 percent, and as much as 250 percent, by 2045 with the exception of Aggregates and Energy products, which are forecasted to grow by 75 percent and decrease by 82 percent, respectively, in value by 2045. The corridor's foreign trade imports in 2015 accounted for the majority of foreign trade volume at 67 percent or 609,000 tons, and value at 56 percent or $\$ 1.9$ billion.


Figure 16. Foreign Trade Commodity Values, 2015 and 2045

Canada was the corridor's top foreign trade partner by volume in 2015 with about 258,000 tons, or 28 percent of the total trade volume, as shown in Figure 17. While Eastern Asia was ranked second in 2015 with 205,000 tons, it is forecasted to be the top trade partner by volume in 2045 with 709,000 tons compared to Canada's 570,000 tons. Europe ranked third by volume with almost 153,000 tons in 2015 and 279,000 tons in 2045. The Rest of the Americas and Southwest and Central Asia are forecasted to increase at a higher volume rate than Europe to reach approximately 266,000 and 270,000 tons, respectively, by 2045.


Figure 17. Foreign Trade Partners by Volume, 2015 and 2045

In 2015, Eastern Asia and Europe were the top ranked trade partner by value, worth over $\$ 1.0$ billion and $\$ 684$ million, respectively. By 2045, the value of goods is forecasted to grow to $\$ 3.2$ billion for Eastern Asia and $\$ 2.1$ billion for Europe. While Canada ranked first in terms of the volume of goods in 2015, it only ranked fourth in value, worth a total of $\$ 338$ million. Trade with Canada is forecasted to be worth $\$ 1.1$ billion in 2045, placing it third behind Eastern Asia and Europe, as shown in Figure 18.


Figure 18. Foreign Trade Partners by Value, 2015 and 2045

## 4. Highway Safety and Environmental Resiliency

U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 is a key multimodal transportation corridor with a principal mobility expectation to provide safe and reliable travel.

### 4.1. Corridor Safety

NCDOT planning level safety scoring data from 2015-2019 was analyzed along U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 to identify areas of potential safety concern. Safety scores are based on three components: the class density ratio, the severity index, and the critical crash rate ratio. ${ }^{4}$ The points from the three safety score components are averaged. Section safety scores are grouped into three point ranges where higher scores are considered to have the poorer highway safety performance. Table 14 shows the number of miles along U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 by safety score. Maps of the planning level safety scores along U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 are included in Appendix B.

Table 14. Planning Level Section Safety Scores along U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 from 2015-2019

| Section Safety Score ${ }^{* *}$ | Length of Corridor per Safety <br> Score (miles) |
| :---: | :---: |
| 0 to 33 | 10.9 |
| 33 to 66 | 22.9 |
| 66 to 100 | 52.9 |
| Less than $60 \%$ Mileposted*** | 0.1 |

*Section safety scores were calculated on existing roadways for the portion of the corridor that is not yet existing or under construction.
**Higher scores are considered to have poorer highway safety performance.
***Routes having a mileposted crash percentage of $60 \%$ or lower were not scored.

### 4.2. Corridor Resiliency

The resiliency of U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 is critical to achieving the goal of providing safe and reliable travel. The resiliency is defined by the corridor's ability to continue to provide service during natural disasters and weather events and to recover from crashes, accidents, and other safety concerns in a timely manner.

Environmental resiliency along the corridor was analyzed using North Carolina Flood Risk Information System (FRIS) floodplain data, historic flood events catalogued in the NCDOT Drive N.C. database from 2011 to 2019, and road inundation incidents. A summary of the FRIS flood zones are shown in Table 15 by the miles of the corridor that are within the floodway, 100-year floodplain, and the 500-year floodplain. Of the corridor's entire length of 101 miles, only about 11 of those miles are within flood zones. Maps of the flood zones along U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 are included in Appendix C .

[^5]Table 15. Flood Zone Summary

| Flood Zone | Length of Corridor in Flood <br> Zone (feet) |
| :--- | :---: |
| Floodway | $10,973.8$ |
| 100 -Year Floodplain | $25,826.2$ |
| 500 -Year Floodplain | $18,898.9$ |
| Total | $55,698.9$ |

Note: total corridor length: 532,237 feet
*Inclusive of floodway
**Inclusive of floodway and 100-year floodplain
Roadway flood incidents that occurred along U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 are shown in Table
16. A total of 19 flood incidents were recorded along U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 from 2011 to 2019. These incidents were caused by Hurricanes Matthew and Florence and resulted in impassable road conditions and instances where afflicted segments of the corridor were closed. Maps of the flood incidents along U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 are included in Appendix C.

Table 16. Flood Incident Summary

| Incident ID | County | Route | Date | Road Condition | Incident Type | Event Name |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 492781 | Pitt | N.C. 11 | $10 / 9 / 2016$ | Road Closed | Weather Event | Hurrricane Matthew |
| 493403 | Lenoir | N.C. 11 | $10 / 10 / 2016$ | Road Closed with Detour | Weather Event | Hurrricane Matthew |
| 493643 | Pitt | N.C. 11 | $10 / 11 / 2016$ | Road Closed | Weather Event | Hurrricane Matthew |
| 493730 | Lenoir | N.C. 11 | $10 / 12 / 2016$ | Road Closed | Weather Event | Hurrricane Matthew |
| 493845 | Lenoir | U.S. 258 | $10 / 12 / 2016$ | Road Closed with Detour | Weather Event | Hurrricane Matthew |
| 505688 | Lenoir | N.C. 11 | $5 / 2 / 2017$ | Road Closed with Detour | Weather Event | None |
| 542596 | Lenoir | N.C. 11 | $9 / 14 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |
| 542728 | Lenoir | N.C. 11 | $9 / 14 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |
| 542798 | Lenoir | U.S. 258 | $9 / 15 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |
| 542803 | Lenoir | U.S. 258 | $9 / 15 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |
| 542818 | Jones | U.S. 258 | $9 / 15 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |
| 543154 | Lenoir | U.S. 258 | $9 / 15 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |
| 543155 | Lenoir | U.S. 258 | $9 / 15 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |
| 543157 | Lenoir | U.S. 258 | $9 / 15 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |
| 543194 | Lenoir | U.S. 258 | $9 / 15 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |
| 543839 | Lenoir | N.C. 11 | $9 / 16 / 2018$ | Road Closed | Weather Event | Hurricane Florence |
| 544733 | Onslow | U.S. 258 | $9 / 18 / 2018$ | Road Closed | Weather Event | Hurricane Florence |
| 545075 | Lenoir | U.S. 258 | $9 / 20 / 2018$ | Road Closed | Weather Event | Hurricane Florence |
| 545316 | Lenoir | N.C. 11 | $9 / 21 / 2018$ | Road Impassable | Weather Event | Hurricane Florence |

*Flood incidents were collected on existing roadways for the portion of the corridor that is not yet existing or under construction.
In addition to the historic flood incidents, road inundation incidents were analyzed along U.S. 13/U.S. 264/N.C. 11/C.F. Harvey Pkwy/U.S. 258 by segment. Segments were defined by clusters of data points for the "100" recurrence interval. Recurrence intervals are the estimated average time between when inundation events caused by flooding are likely to occur; this metric is used for risk analysis. Lower recurrence intervals typically correspond to greater risks of inundation. Road inundation incidents were analyzed at the 10, 25, 50, and 100-year recurrence intervals. Table 17 summarizes the road inundation incidents by each recurrence intervals' average and maximum depths-where depth is the measure of
water flooding a roadway-and the length of the corridor inundated, all per segment. The total of each recurrence interval is also included in the table. Maps of the road inundation incidents along the corridor are included in Appendix C.

Table 17. Road Inundation Incident Summary

| Recurrence Interval** | Route* | Average Depth of Inundation (ft)** | Maximum Depth | Linear Feet Inundated (ft) | Percent of Corridor Inundated |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Segment 1 |  |  |  |  |  |
| 10 | N/A | N/A | N/A | N/A | N/A |
| 25 | N/A | N/A | N/A | N/A | N/A |
| 50 | U.S. 264 | 0.2 | 0.4 | 900 | 0.17\% |
| 100 | U.S. 264 | 1.1 | 1.8 | 2000 | 0.38\% |
| Segment 2 |  |  |  |  |  |
| 10 | N/A | N/A | N/A | N/A | N/A |
| 25 | N/A | N/A | N/A | N/A | N/A |
| 50 | N/A | N/A | N/A | N/A | N/A |
| 100 | N.C. 11 | 0.3 | 0.5 | 1550 | 0.29\% |
| Segment 3 |  |  |  |  |  |
| 10 | N/A | N/A | N/A | N/A | N/A |
| 25 | N/A | N/A | N/A | N/A | N/A |
| 50 | U.S. 258 \& U.S. 70 | 0.2 | 0.4 | 235 | 0.04\% |
| 100 | U.S. 258 \& U.S. 70 | 1.2 | 1.9 | 700 | 0.13\% |
| Segment 4 |  |  |  |  |  |
| 10 | N/A | N/A | N/A | N/A | N/A |
| 25 | N/A | N/A | N/A | N/A | N/A |
| 50 | U.S. 70 \& U.S. 70 BUS | 0.6 | 1.0 | 1100 | 0.21\% |
| 100 | U.S. 70 \& U.S. 70 BUS | 1.8 | 2.4 | 1450 | 0.27\% |
| Segment 5 |  |  |  |  |  |
| 10 | N/A | N/A | N/A | N/A | N/A |
| 25 | N/A | N/A | N/A | N/A | N/A |
| 50 | U.S. 70 | 0.4 | 1.5 | 6150 | 1.16\% |
| 100 | U.S. 70 | 1.5 | 2.9 | 8450 | 1.59\% |
| Segment 6 |  |  |  |  |  |
| 10 | N/A | N/A | N/A | N/A | N/A |
| 25 | N/A | N/A | N/A | N/A | N/A |
| 50 | N/A | N/A | N/A | N/A | N/A |
| 100 | U.S. 70 | 0.4 | 0.8 | 500 | 0.09\% |

[^6]Table 16. Road Inundation Incident Summary (Continued)

| Recurrence Interva\|** | Route* | Average Depth of Inundation (ft)*** | Maximum Depth | Linear Feet Inundated (ft) | Percent of Corridor Inundated |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Segment 7 |  |  |  |  |  |
| 10 | N/A | N/A | N/A | N/A | N/A |
| 25 | N/A | N/A | N/A | N/A | N/A |
| 50 | U.S. 258 | 0.2 | 0.3 | 300 | 0.06\% |
| 100 | U.S. 258, U.S. 70, \& U.S. 70 BUS | 0.9 | 1.6 | 3660 | 0.69\% |
| Segment 8 |  |  |  |  |  |
| 10 | U.S. 258 | 0.4 | 0.6 | 100 | 0.02\% |
| 25 | U.S. 258 | 0.8 | 1.1 | 150 | 0.03\% |
| 50 | U.S. 258 | 0.7 | 1.2 | 250 | 0.05\% |
| 100 | U.S. 258 | 0.9 | 1.3 | 250 | 0.05\% |
| Segment 9 |  |  |  |  |  |
| 10 | N/A | N/A | N/A | N/A | N/A |
| 25 | N/A | N/A | N/A | N/A | N/A |
| 50 | N/A | N/A | N/A | N/A | N/A |
| 100 | N.C. 24 BUS | 0.2 | 0.2 | 150 | 0.03\% |
| Total |  |  |  |  |  |
| 10 | N/A | 0.4 | 0.6 | 100 | 0.02\% |
| 25 | N/A | 0.8 | 1.1 | 150 | 0.03\% |
| 50 | N/A | 0.4 | 1.5 | 8935 | 1.68\% |
| 100 | N/A | 0.9 | 2.9 | 18710 | 3.52\% |

*Road inundation incidents were collected on existing roadways for the portion of the corridor that is not yet existing or under construction.
**Gray represents each segment's design frequency as defined in Table 7-1 of the NCDOT Guidelines for Drainage Studies and Hydraulic Design, included in Appendix D.
***"N/A" indicates that there are no road inundation incidents in a given recurrence interval in the segment.

## Appendix A

NCDOT Level of Service D Standards for Systems and Level Planning



TRANSPGRTATIDNPLANNINGERANCH

## Level of Service D Standards for Systems Level Planning



Driver Comfort: High
Maximum Density:
12 passenger cars per mile per lane


Driver Comfort: Poor
Maximum Density:
42 passenger cars per mile per lane

Level of Service B


Driver Comfort: High
Maximum Density:
20 passenger cars per mile per lane


Driver Comfort: Extremely Poor
Maximum Density:
67 passenger cars per mile per lane

Level of Service C


Driver Comfort: Some Tension Maximum Density:

30 passenger cars per mile cer lane

Level of Service F


Driver Comfort:The lowest
Maximum Density:
More than 67 passenger cars per mile per lane

## General Disclaimer

The Level of Service D Standards for Systems Level Planning was derived from the 2005 North Carolina Level of Service (NCLOS) Version 2.1 Program developed by the Institute for Transportation Research and Education (ITRE) at North Carolina State University. The NCLOS Program is based on the 2000 Highway Capacity Manual, published by the Transportation Research Board (TRB).

These standards are intended for systems level planning only. Many assumptions are made and documented in the development of these standards.

## CTP FACILITY TYPES

FREEWAYS represent a multi-lane divided facility with complete access control (interchanges only and no traffic signals).

EXPRESSWAYS represent a multi-lane divided facility with a high level of access control (interchanges, limited at-grade intersections, right-in/right out access, and no traffic signals).

BOULEVARDS represent a typically divided facility with moderate access control (at-grade intersections, right-in/right out access, and traffic signals at major intersections).

OTHER MAJOR THOROUGHFARES represent undivided facilities with four or more lanes (US and NC routes may have less than 4 lanes). These facilities typically have low access control (at-grade intersections, access to development, and traffic signals at major and some minor intersections).

MINOR THOROUGHFARES represent a 2-to-3 lane undivided facility that is not signed as a US or NC route. These facilities typically have low access control (at-grade intersections, access to development, and traffic signals at major and minor intersections).

## NCLOS (HCM) FACILITY TYPES

FREEWAYS (Freeways) represent a multi-lane divided facility with complete access control (interchanges only and no traffic signals).

EXPRESSWAYS (Multi-lane Highways) represent a multi-lane divided facility with a high level of access control (interchanges, limited at-grade intersections, right-in/right out access, and no traffic signals).

BOULEVARDS (Arterials, 25-55 MPH) represent a typically divided facility with moderate access control (at-grade intersections, rightin/right out access, and traffic signals at major intersections).

OTHER MAJOR THOROUGHFARES (Arterials, 25-55 MPH) represent undivided facilities with four or more lanes (US and NC routes may have less than 4 lanes). These facilities typically have low access control (at-grade intersections, access to development, and traffic signals at major and some minor intersections). These facilities are typically within an urban or suburban area (e.g. within a municipality or ETJ).

MINOR THOROUGHFARES (Arterials 25-55 MPH) represent a 2-to3 lane undivided facility that is not signed as a US or NC route. These facilities typically have low access control (at-grade intersections, access to development, and traffic signals at major and minor intersections). These facilities are typically within an urban or suburban area (e.g. within a municipality or ETJ).

RURAL 2-LANE HIGHWAY (Two-Lane Highway, 55 MPH ONLY) represents a 2-lane undivided facility outside of a municipality or ETJ. These facilities have a 55 MPH posted speed limit, have low access control with numerous driveways and no traffic signals. These facilities are classified in a CTP as other major thoroughfares if they are a US or NC route or minor thoroughfares if they are a secondary or local route.

## AREA TYPE

RURAL represents an area outside a municipality or Extraterritorial Jurisdiction (ETJ).

SUBURBAN represents an area within a municipality or ETJ that is not within a Central Business District (CBD) or areas immediately surrounding a CBD.

URBAN represents an area that is within a CBD or areas immediately surrounding a CBD.

## LEVEL OF SERVICE D VALUES

MINIMUM CAPACITY VALUES represents conditions/inputs that result in a worst-case Level of Service D for a given facility. This lower value represents worst-case conditions in available data for a given region (Higher K/D Factors, Lower Peak Hour Factor, poor road conditions, etc.).

STANDARD CAPACITY VALUES represents an average Level of Service $D$ for a given facility. This default value is an average of available data for a given region.

MAXIMUM CAPACITY VALUES represents conditions/inputs that result in a best-case Level of Service D for a given facility. This higher value represents best-case conditions in available data for a given region (Lower K/D Factors, Higher Peak Hour Factor, etc.).

These assumptions may not pertain to all systems level planning work; therefore, separate analysis may need to be conducted on a case-by-case basis.

These standards are not intended for project specific or corridor analysis. Separate analysis would be required for these types of projects.

Volumes shown represent the point at which traffic transitions from LOS D to LOS E.

## Level of Service D Standards for Freeways *

| cOASTAL | 2 Lanes Per Direction |  | 3 Lanes Per Direction |  |  | 4 Lanes Per Direction |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| $0-5 \%$ Trucks | 67400 | 66900 | 67900 |  | 102000 | 101300 | 101800 | 137300 | 136200 | 135700 |
| $6-10 \%$ Trucks | 65700 | 65400 | 66200 |  | 99600 | 98900 | 99400 | 134000 | 133000 | 132500 |
| $11-15 \%$ Trucks | 64200 | 63800 | 64700 |  | 97300 | 96600 | 97100 | 130900 | 129900 | 129400 |
| $16-20 \%$ Trucks | 62800 | 62400 | 63200 |  | 95100 | 94400 | 94900 | 127900 | 126900 | 126500 |
| $21-25 \%$ Trucks | 61400 | 61000 | 61800 |  | 9300 | 92300 | 92700 | 125100 | 124100 | 123700 |
| $26-30 \%$ Trucks | 60000 | 59700 | 60500 |  | 90900 | 90300 | 90700 | 122400 | 121400 | 121000 |
| $31-35 \%$ Trucks | 58800 | 58400 | 59200 |  | 89000 | 88400 | 88800 | 119800 | 118800 | 118400 |


| PIEDMONT | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  | 4 Lanes Per Direction |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 0-5\% Trucks | 61700 | 61400 | 62200 | 93500 | 92900 | 93300 | 125800 | 124900 | 124400 |
| 6-10\% Trucks | 60300 | 59900 | 60700 | 91300 | 90700 | 91100 | 122800 | 121900 | 121500 |
| 11-15\% Trucks | 58900 | 58500 | 59300 | 89200 | 88600 | 89000 | 120000 | 119100 | 118600 |
| 16-20\% Trucks | 57500 | 57200 | 58000 | 87100 | 86500 | 87000 | 117300 | 116400 | 115900 |
| 21-25\% Trucks | 56300 | 55900 | 56700 | 85200 | 84600 | 85000 | 114700 | 113800 | 113400 |
| 26-30\% Trucks | 55000 | 54700 | 55400 | 83400 | 82800 | 83200 | 112200 | 111300 | 110900 |
| 31-35\% Trucks | 53900 | 53500 | 54300 | 81600 | 81000 | 81400 | 109800 | 108900 | 108500 |


| MOUNTAIN <br> (Level Terrain) | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  | 4 Lanes Per Direction |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| $0-5 \%$ Trucks | 56100 | 61400 | 62200 |  | 85000 | 92900 | 93300 | 114400 | 124900 | 124400 |
| $6-10 \%$ Trucks | 54800 | 59900 | 60700 |  | 83000 | 90700 | 91100 | 111700 | 121900 | 121500 |
| $11-15 \%$ Trucks | 53500 | 58500 | 59300 |  | 81100 | 88600 | 89000 | 109100 | 119100 | 118600 |
| $16-20 \%$ Trucks | 52300 | 57200 | 58000 |  | 79200 | 86500 | 87000 | 106600 | 116400 | 115900 |
| $21-25 \%$ Trucks | 51100 | 55900 | 56700 |  | 77500 | 84600 | 85000 | 104200 | 113800 | 113400 |
| $26-30 \%$ Trucks | 50000 | 54700 | 55400 |  | 75800 | 82800 | 83200 | 102000 | 111300 | 110900 |
| $31-35 \%$ Trucks | 49000 | 53500 | 54300 |  | 74200 | 81000 | 81400 | 99800 | 108900 | 108500 |


| MOUNTAIN(Rolling Terrian) | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  | 4 Lanes Per Direction |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 0-5\% Trucks | 53500 | 58500 | 59300 | 81100 | 88600 | 89000 | 109100 | 119100 | 118600 |
| 6-10\% Trucks | 50000 | 54700 | 55400 | 75800 | 82800 | 83200 | 102000 | 111300 | 110900 |
| 11-15\% Trucks | 47000 | 51400 | 52100 | 71100 | 77700 | 78100 | 95700 | 104500 | 104100 |
| 16-20\% Trucks | 44300 | 48400 | 49000 | 67000 | 73200 | 73600 | 90200 | 98500 | 98100 |
| 21-25\% Trucks | 41800 | 45700 | 46400 | 63400 | 69200 | 69600 | 85300 | 93100 | 92700 |
| 26-30\% Trucks | 39700 | 43400 | 44000 | 60100 | 65700 | 66000 | 80900 | 88300 | 87900 |
| 31-35\% Trucks | 37700 | 41200 | 41800 | 57100 | 62400 | 62700 | 76900 | 83900 | 83600 |

## Uses "Freeways" Facility Type in NCLOS <br> * Assumes Regional K and D Factor Averages

See Appendix A1 for HCM 2000 Freeway Equations
Use Appendix A2: Coastal Freeway Inputs for adjustments
Use Appendix A3: Piedmont Freeway Inputs for adjustments
Use Appendix A4: Mountain (Level) Freeway Inputs for adjustments
Use Appendix A5: Mountain (Rolling) Freeway Inputs for adjustments
NOTE: Truck percentage occurs within the peak hour, not a daily truck percentage

# Level of Service D Standards for Expressways * 

| COASTAL | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  | 4 Lanes Per Direction |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 0-5\% Trucks | 47500 | 58500 | 58800 | 71200 | 87700 | 88300 | 95000 | 117000 | 117700 |
| 6-10\% Trucks | 46400 | 57100 | 57400 | 69500 | 85600 | 86200 | 92700 | 114200 | 114900 |
| 11-15\% Trucks | 45300 | 55800 | 56100 | 67900 | 83700 | 84200 | 90600 | 111500 | 112200 |
| 16-20\% Trucks | 44200 | 54500 | 54800 | 66400 | 81800 | 82200 | 88500 | 109000 | 109700 |
| 21-25\% Trucks | 43300 | 53300 | 53600 | 64900 | 79900 | 80400 | 86500 | 106600 | 107200 |
| 26-30\% Trucks | 42300 | 52100 | 52400 | 63500 | 78200 | 78700 | 84700 | 104300 | 104900 |
| 31-35\% Trucks | 41400 | 51000 | 51300 | 62100 | 76500 | 77000 | 82900 | 102100 | 102700 |


| PIEDMONT | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  | 4 Lanes Per Direction |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 0-5\% Trucks | 47500 | 58500 | 58800 | 71200 | 87700 | 88300 | 95000 | 117000 | 117700 |
| 6-10\% Trucks | 46400 | 57100 | 57400 | 69500 | 85600 | 86200 | 92700 | 114200 | 114900 |
| 11-15\% Trucks | 45300 | 55800 | 56100 | 67900 | 83700 | 84200 | 90600 | 111500 | 112200 |
| 16-20\% Trucks | 44200 | 54500 | 54800 | 66400 | 81800 | 82200 | 88500 | 109000 | 109700 |
| 21-25\% Trucks | 43300 | 53300 | 53600 | 64900 | 79900 | 80400 | 86500 | 106600 | 107200 |
| 26-30\% Trucks | 42300 | 52100 | 52400 | 63500 | 78200 | 78700 | 84700 | 104300 | 104900 |
| 31-35\% Trucks | 41400 | 51000 | 51300 | 62100 | 76500 | 77000 | 82900 | 102100 | 102700 |


| $\begin{gathered} \text { MOUNTAIN } \\ \text { (Level Terrain) } \end{gathered}$ | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  | 4 Lanes Per Direction |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 0-5\% Trucks | 47500 | 53200 | 58800 | 71200 | 79800 | 88300 | 95000 | 106400 | 117700 |
| 6-10\% Trucks | 46400 | 51900 | 57400 | 69500 | 77900 | 86200 | 92700 | 103800 | 114900 |
| 11-15\% Trucks | 45300 | 50700 | 56100 | 67900 | 76100 | 84200 | 90600 | 101400 | 112200 |
| 16-20\% Trucks | 44200 | 49500 | 54800 | 66400 | 74300 | 82200 | 88500 | 99100 | 109700 |
| 21-25\% Trucks | 43300 | 48400 | 53600 | 64900 | 72700 | 80400 | 86500 | 96900 | 107200 |
| 26-30\% Trucks | 42300 | 47400 | 52400 | 63500 | 71100 | 78700 | 84700 | 94800 | 104900 |
| 31-35\% Trucks | 41400 | 46400 | 51300 | 62100 | 69600 | 77000 | 82900 | 92800 | 102700 |


| MOUNTAIN (Rolling Terrian) | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  | 4 Lanes Per Direction |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 0-5\% Trucks | 41200 | 50700 | 56100 | 61700 | 76100 | 84200 | 82300 | 101400 | 112200 |
| 6-10\% Trucks | 38500 | 47400 | 52400 | 57700 | 71100 | 78700 | 77000 | 94800 | 110400 |
| 11-15\% Trucks | 36100 | 44500 | 49200 | 54200 | 66700 | 73900 | 72200 | 89000 | 98500 |
| 16-20\% Trucks | 34000 | 41900 | 46400 | 51100 | 62900 | 69600 | 68100 | 83900 | 92800 |
| 21-25\% Trucks | 32200 | 39600 | 43900 | 48300 | 59500 | 65800 | 64400 | 79300 | 87700 |
| 26-30\% Trucks | 30500 | 37600 | 41600 | 45800 | 56400 | 62400 | 61000 | 75200 | 83200 |
| 31-35\% Trucks | 29000 | 35700 | 39600 | 43500 | 53600 | 59300 | 58000 | 71500 | 79100 |

## Uses "Multi-lane Highways" Facility Type in NCLOS

* Assumes Regional K and D Factor Averages

See Appendix B1 for HCM 2000 Multi-lane Highway Equations
Use Appendix B2: Coastal Expressway Inputs for adjustments
Use Appendix B3: Piedmont Expressway Inputs for adjustments
Use Appendix B4: Mountain (Level) Expressway Inputs for adjustments
Use Appendix B5: Mountain (Rolling) Expressway Inputs for adjustments

NOTE: Truck percentage occurs within the peak hour, not a daily truck percentage

## Level of Service D Standards for Boulevards *

| COASTAL | 1 Lane Per Direction |  |  | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 55 MPH | 21600 | 21900 | 24500 | 43300 | 43900 | 49000 | 64900 | 65800 | 73500 |
| 45 MPH | 18900 | 19800 | 23600 | 38100 | 39700 | 47200 | 57200 | 59600 | 70800 |
| 35 MPH | 14000 | 16900 |  | 28100 | 34300 |  | 42200 | 51700 |  |
| 25 MPH | 12500 |  |  | 25400 |  |  | 38400 |  |  |


| PIEDMONT | 1 Lane Per Direction |  |  | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 55 MPH | 19900 | 20200 | 22600 | 40000 | 40500 | 45200 | 59900 | 60700 | 67900 |
| 45 MPH | 17500 | 18300 | 21800 | 35100 | 36600 | 43600 | 52800 | 55000 | 65400 |
| 35 MPH | 14000 | 15600 |  | 28100 | 31600 |  | 42200 | 47700 |  |
| 25 MPH | 12500 |  |  | 25400 |  |  | 38400 |  |  |


| MOUNTAIN | 1 Lane Per Direction |  |  | 2 Lanes Per Direction |  |  | 3 Lanes Per Direction |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 55 MPH | 21600 | 21900 | 22300 | 43300 | 43900 | 44500 | 64900 | 65800 | 66800 |
| 45 MPH | 18900 | 20700 | 21400 | 38100 | 41400 | 42900 | 57200 | 62100 | 64400 |
| 35 MPH | 14000 | 18500 |  | 28100 | 37400 |  | 42200 | 56400 |  |
| 25 MPH | 12500 |  |  | 25400 |  |  | 38400 |  |  |

Uses "Principal Arterials" Facility Type in NCLOS

* Assumes Regional K and D Factor Averages


## See Appendix C1 for HCM Urban Arterial Equations

Use Appendix C2: Coastal Boulevard Inputs for adjustments
Use Appendix C3: Piedmont Boulevard Inputs for adjustments
Use Appendix C4: Mountain Boulevard Inputs for adjustments
NOTE: Inputs assume 12-foot lanes. To adjust lane-width downward, subtract $3.33 \%$ per foot of pavement and round to the nearest hundred

# Coastal Level of Service D Standards for Other Major Thoroughfares * 

| 55 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 15100 | 15800 | 16400 | 16600 | 17200 | 17800 |
| 11 foot lanes | 14600 | 15300 | 15900 | 16100 | 16600 | 17200 |
| 10 foot lanes | 14100 | 14700 | 15300 | 15500 | 16100 | 16600 |
| 9 foot lanes | 13600 | 14200 | 14800 | 15000 | 15500 | 16000 |
| 45 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 13200 | 13800 | 14600 | 14500 | 14900 | 16000 |
| 11 foot lanes | 12800 | 13300 | 14100 | 14000 | 14400 | 15500 |
| 10 foot lanes | 12300 | 12900 | 13600 | 13500 | 13900 | 15000 |
| 9 foot lanes | 11900 | 12420 | 13140 | 13050 | 13400 | 14400 |
| 35 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 11100 | 12600 |  | 12700 | 14000 |  |
| 11 foot lanes | 10700 | 12200 |  | 12300 | 13500 |  |
| 10 foot lanes | 10400 | 11800 |  | 11900 | 13100 |  |
| 9 foot lanes | 10000 | 11300 |  | 11400 | 12600 |  |
| 25 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 11000 |  |  | 12700 |  |  |
| 11 foot lanes | 10600 |  |  | 12300 |  |  |
| 10 foot lanes | 10300 |  |  | 11900 |  |  |
| 9 foot lanes | 9900 |  |  | 11400 |  |  |

Uses "Principal Arterials" Facility Type in NCLOS

* Decrease in Lane Width Capacity calculated via 2000 Highway Capacity Manual lane-width adjustment factor for saturation flow rate

See Appendix D1 for HCM 2000 Urban Arterial Equations
Use Appendix D2: Coastal Major Thoroughfare Inputs for adjustments
NOTE: Lane Width is adjusted downward by $3.33 \%$ per less foot of pavement and rounded to the nearest hundred

# Coastal Level of Service D Standards for Other Major Thoroughfares * 

| 55 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 30400 | 31600 | 32800 | 33300 | 34500 | 35700 |
| 11 foot lanes | 29400 | 30600 | 31700 | 32200 | 33400 | 34500 |
| 10 foot lanes | 29400 | 29500 | 30600 | 31100 | 32200 | 33300 |
| 9 foot lanes | 27400 | 28400 | 29500 | 30000 | 31100 | 32100 |
| 45 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 26700 | 27600 | 29300 | 29000 | 29900 | 32000 |
| 11 foot lanes | 25900 | 26700 | 28300 | 28000 | 28900 | 30900 |
| 10 foot lanes | 25000 | 25800 | 27300 | 27100 | 27900 | 29900 |
| 9 foot lanes | 24000 | 24800 | 26400 | 26100 | 26900 | 29000 |
| 35 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 22200 | 25500 |  | 24300 | 28100 |  |
| 11 foot lanes | 21500 | 24700 |  | 23500 | 27200 |  |
| 10 foot lanes | 20700 | 23800 |  | 22700 | 26200 |  |
| 9 foot lanes | 20000 | 23000 |  | 21900 | 25300 |  |
| 25 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 22100 |  |  | 24200 |  |  |
| 11 foot lanes | 21400 |  |  | 23400 |  |  |
| 10 foot lanes | 20500 |  |  | 22600 |  |  |
| 9 foot lanes | 19900 |  |  | 21800 |  |  |

Uses "Principal Arterials" Facility Type in NCLOS

* Decrease in Lane Width Capacity calculated via 2000 Highway Capacity Manual lane-width adjustment factor for saturation flow rate

See Appendix D1 for HCM 2000 Urban Arterial Equations
Use Appendix D2: Coastal Major Thoroughfare Inputs for adjustments
NOTE: Lane Width is adjusted downward by $3.33 \%$ per less foot of pavement and rounded to the nearest hundred

# Piedmont Level of Service D Standards for Other Major Thoroughfares * 

| 55 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 12900 | 14600 | 15100 | 14200 | 15900 | 16500 |
| 11 foot lanes | 12500 | 14100 | 14600 | 13700 | 15400 | 16000 |
| 10 foot lanes | 12000 | 13600 | 14100 | 13300 | 14800 | 15400 |
| 9 foot lanes | 11600 | 13100 | 13600 | 12800 | 14300 | 14900 |
| 45 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 12200 | 12700 | 14600 | 13300 | 13800 | 16000 |
| 11 foot lanes | 11800 | 12300 | 14100 | 12900 | 13300 | 15500 |
| 10 foot lanes | 11400 | 11900 | 13600 | 12400 | 12900 | 14900 |
| 9 foot lanes | 11000 | 11400 | 13100 | 12000 | 12400 | 14400 |
| 35 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 11100 | 11600 |  | 12700 | 12900 |  |
| 11 foot lanes | 10700 | 11200 |  | 12300 | 12500 |  |
| 10 foot lanes | 10400 | 10800 |  | 11900 | 12000 |  |
| 9 foot lanes | 10000 | 10400 |  | 11400 | 11600 |  |
| 25 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 11000 |  |  | 12700 |  |  |
| 11 foot lanes | 10600 |  |  | 12300 |  |  |
| 10 foot lanes | 10300 |  |  | 11900 |  |  |
| 9 foot lanes | 9900 |  |  | 11400 |  |  |

Uses "Principal Arterials" Facility Type in NCLOS

* Decrease in Lane Width Capacity calculated via 2000 Highway Capacity Manual lane-width adjustment factor for saturation flow rate

See Appendix D1 for HCM 2000 Urban Arterial Equations
Use Appendix D3: Piedmont Major Thoroughfare Inputs for adjustments
NOTE: Lane Width is adjusted downward by $3.33 \%$ per less foot of pavement

# Piedmont Level of Service D Standards for Other Major Thoroughfares * 

| 55 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 25800 | 29100 | 30200 | 28400 | 31800 | 33000 |
| 11 foot lanes | 24900 | 28100 | 29200 | 27500 | 30800 | 31900 |
| 10 foot lanes | 24100 | 27200 | 28200 | 26500 | 29700 | 30800 |
| 9 foot lanes | 23200 | 26200 | 27200 | 25600 | 28600 | 29700 |
| 45 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 24600 | 25500 | 29300 | 26800 | 27600 | 32000 |
| 11 foot lanes | 23800 | 24700 | 28300 | 25900 | 26700 | 31000 |
| 10 foot lanes | 23000 | 23800 | 27300 | 25000 | 25800 | 29900 |
| 9 foot lanes | 22100 | 23000 | 26400 | 24100 | 24800 | 28800 |
| 35 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 22200 | 23500 |  | 24300 | 26000 |  |
| 11 foot lanes | 21500 | 22700 |  | 23500 | 25100 |  |
| 10 foot lanes | 20700 | 21900 |  | 22700 | 24300 |  |
| 9 foot lanes | 20000 | 21200 |  | 21900 | 23400 |  |
| 25 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 22100 |  |  | 24200 |  |  |
| 11 foot lanes | 21400 |  |  | 23400 |  |  |
| 10 foot lanes | 20600 |  |  | 22600 |  |  |
| 9 foot lanes | 19900 |  |  | 21800 |  |  |

Uses "Principal Arterials" Facility Type in NCLOS

* Decrease in Lane Width Capacity calculated via 2000 Highway Capacity Manual lane-width adjustment factor for saturation flow rate

See Appendix D1 for HCM 2000 Urban Arterial Equations
Use Appendix D3: Piedmont Major Thoroughfare Inputs for adjustments
NOTE: Lane Width is adjusted downward by $3.33 \%$ per less foot of pavement and rounded to the nearest hundred

# Mountain Level of Service D Standards for Other Major Thoroughfares * 

| 55 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 14000 | 14600 | 15100 | 15300 | 15900 | 16500 |
| 11 foot lanes | 13500 | 14100 | 14600 | 14800 | 15400 | 16000 |
| 10 foot lanes | 13100 | 13600 | 14100 | 14300 | 14800 | 15400 |
| 9 foot lanes | 12600 | 13100 | 13600 | 13800 | 14300 | 14900 |
| 45 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 12200 | 12700 | 14600 | 13300 | 13800 | 16000 |
| 11 foot lanes | 11800 | 12300 | 14100 | 12900 | 13300 | 15500 |
| 10 foot lanes | 11400 | 11900 | 13600 | 12400 | 12900 | 14900 |
| 9 foot lanes | 11000 | 11400 | 13100 | 12000 | 12400 | 14400 |
| 35 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 11000 | 11600 |  | 12700 | 12900 |  |
| 11 foot lanes | 10600 | 11200 |  | 12300 | 12500 |  |
| 10 foot lanes | 10300 | 10800 |  | 11900 | 12000 |  |
| 9 foot lanes | 9900 | 10400 |  | 11400 | 11600 |  |
| 25 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 11000 |  |  | 12700 |  |  |
| 11 foot lanes | 10600 |  |  | 12300 |  |  |
| 10 foot lanes | 10300 |  |  | 11900 |  |  |
| 9 foot lanes | 9900 |  |  | 11400 |  |  |

Uses "Principal Arterials" Facility Type in NCLOS

* Decrease in Lane Width Capacity calculated via 2000 Highway Capacity Manual lane-width adjustment factor for saturation flow rate

See Appendix D1 for HCM 2000 Urban Arterial Equations
Use Appendix D4: Mountains Major Thoroughfare Inputs for adjustments
NOTE: Lane Width is adjusted downward by $3.33 \%$ per less foot of pavement and rounded to the nearest hundred

# Mountain Level of Service D Standards for Other Major Thoroughfares * 

| 55 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 28000 | 29100 | 30200 | 30800 | 31800 | 33000 |
| 11 foot lanes | 27100 | 28100 | 29200 | 29800 | 30800 | 31900 |
| 10 foot lanes | 26100 | 27200 | 28200 | 28700 | 29700 | 30800 |
| 9 foot lanes | 25200 | 26200 | 27200 | 27700 | 28600 | 29700 |
| 45 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 24600 | 25500 | 29300 | 26800 | 27600 | 32000 |
| 11 foot lanes | 23800 | 24700 | 28300 | 25900 | 26700 | 30900 |
| 10 foot lanes | 23000 | 23800 | 27300 | 25000 | 25800 | 29900 |
| 9 foot lanes | 22100 | 23000 | 26400 | 24100 | 24800 | 28800 |
| 35 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 22200 | 23500 |  | 24300 | 26000 |  |
| 11 foot lanes | 21500 | 22700 |  | 23500 | 25400 |  |
| 10 foot lanes | 20700 | 21900 |  | 22700 | 24300 |  |
| 9 foot lanes | 20000 | 21200 |  | 21900 | 23400 |  |
| 25 MPH | 2 Lanes Per Direction |  |  | 2 Lanes Per Direction WCLTL |  |  |
|  | Urban | Suburban | Rural | Urban | Suburban | Rural |
| 12 foot lanes | 22100 |  |  | 24200 |  |  |
| 11 foot lanes | 21400 |  |  | 23400 |  |  |
| 10 foot lanes | 20600 |  |  | 22600 |  |  |
| 9 foot lanes | 19900 |  |  | 21800 |  |  |

Uses "Principal Arterials" Facility Type in NCLOS

* Decrease in Lane Width Capacity calculated via 2000 Highway Capacity Manual lane-width adjustment factor for saturation flow rate

See Appendix D1 for HCM 2000 Urban Arterial Equations
Use Appendix D4: Mountains Major Thoroughfare Inputs for adjustments
NOTE: Lane Width is adjusted downward by $3.33 \%$ per less foot of pavement

# Coastal Level of Service D Standards for Minor Thoroughfares * 

| 55 MPH | 1 Lane Per Direction |  |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 15100 | 15800 | 16400 |  | 16600 | 17200 | 17800 |
| 11 foot lanes | 14600 | 15300 | 15900 |  | 16100 | 16600 | 17200 |
| 10 foot lanes | 14100 | 14700 | 15300 |  | 15500 | 16100 | 16600 |
| 9 foot lanes | 13600 | 14200 | 14800 |  | 14900 | 15500 | 16000 |


| 45 MPH | 1 Lane Per Direction |  |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 12700 | 13300 | 14600 |  | 14200 | 14300 | 16000 |
| 11 foot lanes | 12300 | 12900 | 14100 |  | 13700 | 13800 | 15500 |
| 10 foot lanes | 11900 | 12400 | 13600 |  | 13300 | 13300 | 14900 |
| 9 foot lanes | 11400 | 12000 | 13100 |  | 12800 | 12900 | 14400 |


| 35 MPH | 1 Lane Per Direction |  |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 10500 | 11000 |  |  | 11500 | 13700 |  |
| 11 foot lanes | 10200 | 10600 |  |  | 11100 | 13300 |  |
| 10 foot lanes | 9800 | 10300 |  |  | 10700 | 12800 |  |
| 9 foot lanes | 9500 | 9900 |  |  | 10400 | 12300 |  |


| 25 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 10000 |  |  |  | 11300 |  |  |
| 11 foot lanes | 9700 |  |  |  | 10900 |  |  |
| 10 foot lanes | 9300 |  |  |  | 10500 |  |  |
| 9 foot lanes | 9000 |  |  |  | 10200 |  |  |

Uses "Principal Arterials" and "Minor Arterials" Facility Types in NCLOS

* Decrease in Lane Width Capacity calculated via 2000 Highway Capacity Manual lane-width adjustment factor for saturation flow rate

See Appendix E1 for HCM 2000 Urban Arterial Equations
Use Appendix E2: Coastal Minor Thoroughfare Inputs for adjustments
NOTE: Lane Width is adjusted downward by 3.33\% per less foot of pavement

# Piedmont Level of Service D Standards for Minor Thoroughfares * 

| 55 MPH | 1 Lane Per Direction |  |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 12900 | 14600 | 15100 |  | 14200 | 15900 | 16500 |
| 11 foot lanes | 12500 | 14100 | 14600 |  | 13700 | 15400 | 16000 |
| 10 foot lanes | 12000 | 13600 | 14100 |  | 13300 | 14800 | 15400 |
| 9 foot lanes | 11600 | 13100 | 13600 |  | 12800 | 14300 | 14900 |


| 45 MPH | 1 Lane Per Direction |  |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 11700 | 12200 | 14600 |  | 13100 | 13200 | 16000 |
| 11 foot lanes | 11300 | 11800 | 14100 |  | 12700 | 12800 | 15500 |
| 10 foot lanes | 10900 | 11400 | 13600 |  | 12200 | 12300 | 14900 |
| 9 foot lanes | 10500 | 11000 | 13100 |  | 11800 | 11900 | 14400 |


| 35 MPH | L Lane Per Direction |  |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 10200 | 10200 |  |  | 11700 | 12700 |  |
| 11 foot lanes | 9900 | 9900 |  |  | 11300 | 12300 |  |
| 10 foot lanes | 9500 | 9500 |  |  | 10900 | 11900 |  |
| 9 foot lanes | 9200 | 9200 |  |  | 10500 | 11400 |  |


| 25 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 10000 |  |  |  | 11300 |  |  |
| 11 foot lanes | 9700 |  |  |  | 10900 |  |  |
| 10 foot lanes | 9300 |  |  |  | 10500 |  |  |
| 9 foot lanes | 9000 |  |  |  | 10200 |  |  |

Uses "Principal Arterials" and "Minor Arterials" Facility Types in NCLOS

* Decrease in Lane Width Capacity calculated via 2000 Highway Capacity Manual lane-width adjustment factor for saturation flow rate

See Appendix E1 for HCM 2000 Urban Arterial Equations
Use Appendix E3: Piedmont Minor Thoroughfare Inputs for adjustments

NOTE: Lane Width is adjusted downward by 3.33\% per less foot of pavement

# Mountain Level of Service D Standards for Minor Thoroughfares * 

| 55 MPH | 1 Lane Per Direction |  |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 14000 | 14600 | 15100 |  | 15300 | 15900 | 16500 |
| 11 foot lanes | 13500 | 14100 | 14600 |  | 14800 | 15400 | 16000 |
| 10 foot lanes | 13100 | 13600 | 14100 |  | 14300 | 14800 | 15400 |
| 9 foot lanes | 12600 | 13100 | 13600 |  | 13800 | 14300 | 14900 |


| 45 MPH | 1 Lane Per Direction |  |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 11700 | 12200 | 14600 |  | 13100 | 13200 | 16000 |
| 11 foot lanes | 11300 | 11800 | 14100 |  | 12700 | 12800 | 15500 |
| 10 foot lanes | 10900 | 11400 | 13600 |  | 12200 | 12300 | 14900 |
| 9 foot lanes | 10500 | 11000 | 13100 |  | 11800 | 11900 | 14400 |


| 35 MPH | Lane Per Direction |  |  |  | 1 Lane Per Direction WCLTL |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 10200 | 10200 |  |  | 11500 | 12700 |  |
| 11 foot lanes | 9900 | 9900 |  |  | 11100 | 12300 |  |
| 10 foot lanes | 9500 | 9500 |  |  | 10700 | 11900 |  |
| 9 foot lanes | 9200 | 9200 |  |  | 10400 | 11400 |  |


| 25 MPH | 1 Lane Per Direction |  |  | 1 Lane Per Direction WCLTL |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Urban | Suburban | Rural |  | Urban | Suburban | Rural |
| 12 foot lanes | 10000 |  |  |  | 11300 |  |  |
| 11 foot lanes | 9700 |  |  |  | 10900 |  |  |
| 10 foot lanes | 9300 |  |  |  | 10500 |  |  |
| 9 foot lanes | 9000 |  |  |  | 10200 |  |  |

Uses "Principal Arterials" and "Minor Arterials" Facility Types in NCLOS

* Decrease in Lane Width Capacity calculated via 2000 Highway Capacity Manual lane-width adjustment factor for saturation flow rate

See Appendix E1 for HCM 2000 Urban Arterial Equations Use Appendix E4: Mountain Minor Thoroughfare Inputs for adjustments

NOTE: Lane Width is adjusted downward by 3.33\% per less foot of pavement

# Level of Service D Standards for Rural 2-Lane Highways 

| Coastal 2-Lane | COASTAL |  |  |
| :---: | :---: | :---: | :---: |
| Highway Standard | Minimum | Standard | Maximum |
| 12-Foot Lanes | 10500 | $12700^{\star}$ |  |
| 11-Foot Lanes | 10000 |  |  |
| 10-Foot Lanes | 9200 | 12000 |  |
| 9-Foot Lanes | 7700 | 10700 |  |


| Piedmont 2-Lane | PIEDMONT |  |  |
| :---: | :---: | :---: | :---: |
| Highway Standard | Minimum | Standard | Maximum |
| 12-Foot Lanes | 10300 | $12400^{\star}$ |  |
| 11-Foot Lanes | 9900 |  |  |
| 10-Foot Lanes | 9000 | 11800 |  |
| 9-Foot Lanes | 7500 | 10500 |  |


| Mountain 2-Lane | MOUNTAINS (Level) |  |  |
| :---: | :---: | :---: | :---: |
| Highway Standard | Minimum | Standard | Maximum |
| 12-Foot Lanes | 10200 | $12100^{*}$ |  |
| 11-Foot Lanes | 9800 |  |  |
| 10-Foot Lanes | 8800 | 11700 |  |
| 9-Foot Lanes | 7400 | 10300 |  |


| Mountain 2-Lane | MOUNTAINS (Rolling) |  |  |
| :---: | :---: | :---: | :---: |
| Highway Standard | Minimum | Standard | Maximum |
| 12-Foot Lanes | 9600 | $12100^{\star}$ |  |
| 11-Foot Lanes | 9100 |  |  |
| 10-Foot Lanes | 8200 | 11100 |  |
| 9-Foot Lanes | 6300 | 9800 |  |

Uses "2-Lane Highways" Facility Type in NCLOS

* All capacities calculated based on HCM 2000 procedures using HCS software. Under some conditions, two-lane highway capacity is not affected by lane width. This occurs where capacity is governed by Percent Time Spent Following rather than by Average Travel Speed.
\# Best-case/Maximum conditions are less likely to occur where lane widths are below 11 feet. Use caution before selecting "Maximum" values for 9 -ft or $\mathbf{1 0 - f t}$ lanes.

See Appendix F1 for HCM 2000 2-Lane Highway Equations
Use Appendix F2: Coastal Rural 2-Lane Highway Inputs for adjustments
Use Appendix F3: Piedmont Rural 2-Lane Highway Inputs for adjustments
Use Appendix F4: Mountain (Level) Rural 2-Lane Highway Inputs for adjustments
Use Appendix F5: Mountain (Rolling) Rural 2-Lane Highway Inputs for adjustments

## Appendix B <br> Safety Data



 Routes having a mileposted crash percentage of $60 \%$ or lower were not scored Routes having a mileposted crash percentage of $60 \%$ or lower were not scored.

 Routes having a mileposted crash percentage of $60 \%$ or lower were not scored Routes having a mileposted crash percentage of $60 \%$ or lower were not scored.

 Routes having a mileposted crash percentage of $60 \%$ or lower were not scored Routes having a mileposted crash percentage of $60 \%$ or lower were not scored.
Safety data is only displayed on STC Corridor P and relevant crossroads and parallel routes.

## Appendix C

Resiliency Data








## Appendix D

NCDOT Guidelines for Drainage Studies and Hydraulic Design Chapter 7


NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

## GUIDELINES FOR DRAINAGE STUDIES <br> AND <br> HYDRAULIC DESIGN



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STATE HYDRAULICS ENGINEER

## 7 HYDROLOGY

### 7.1 Introduction

The hydrologic analysis phase involves the determination of discharge rates and volumes of runoff that drainage facilities will be required to convey. Acceptable hydrologic methods for highway drainage studies and applicable criteria for their use are discussed in this chapter. When the project site involves a FEMA-regulated stream, discharge methods and values provided in the effective published Flood Insurance Study (FIS) report should be used for determining compliance with National Flood Insurance Program (NFIP) regulations (29). (This may result in the need for additional hydraulic modeling to meet NCDOT design criteria, so there may be both a model for NFIP compliance as well as a design model for the NCDOT project.) The results from any hydrological procedure should be calibrated with historical site information. The design engineer should also consider potential future land use changes within a watershed over the life of a roadway structure and include this effect when estimating design discharges.

### 7.2 Drainage Area Determination

There are a variety of sources for obtaining drainage area data, including USGS topographic contour maps, published lists of drainage areas from study reports (such as FEMA Flood Insurance Studies and USGS water data reports), archived NCDOT Bridge and Culvert Survey and Hydraulic Design Reports (BSR, CSR; Appendix E), digital elevation data (such as Light Detection and Ranging, or LiDAR, data), and the relatively new USGS StreamStats web-based GIS application for North Carolina, which utilizes Digital Elevation Models (DEMs) based on LiDAR data and a combination of local resolution stream data and National Hydrography Datasets (NHD) for automated computation of drainage areas (and other basin characteristics). Drainage areas should be verified during project field review. The design engineer of record is responsible for verifying the accuracy of the drainage area regardless of the method used to obtain it.

### 7.2.1 USGS StreamStats

StreamStats is a web-based GIS application (http://water.usgs.gov/osw/streamstats/north_carolina.html) that was released by USGS in 2012. It allows users to easily obtain streamflow statistics, basin characteristics, etc., for USGS gage data collection stations and for user-selected ungaged locations. The application will delineate the drainage area at user-selected stream locations. The website includes comprehensive instructions and associated help files (including Getting Started and Quick Tour links). Users are advised to review and familiarize themselves with this information before attempting to use the application.

### 7.2.2 USGS Quadrangle Maps

USGS topographic mapping is available through the National Map Viewer website http://nationalmap.gov. Additionally, a GIS web map service (WMS) called USA_Topo_Maps provides a base map of national coverage of USGS topographic contour mapping.

### 7.2.3 Digital Elevation Data

Several sources of digital elevation data are available. The primary and most current, accurate, and readily available data is in the MicroStation TIN (triangular irregular network) file (supplied by NCDOT Location \& Surveys and Photogrammetry Units) for the specific project area. However, this coverage is often inadequate for hydrologic studies, so it may need to be supplemented with other digital elevation data sources, such as LiDAR coverage or USGS Digital Elevation Models. Further details on each of these are discussed below.

### 7.2.3.1 MicroStation TIN Files

NCDOT's Location and Surveys Unit and Photogrammetry Unit collaborate to produce the final survey files for NCDOT projects, including planimetric mapping, digital terrain models (DTMs), and associated TIN files. The DTM file is first generated from processing the raw survey data; then, the DTM file is used to generate a TIN file to represent the existing ground surface. Often, the original TIN files provided for a project do not provide adequate geographical coverage for hydrologic analyses (e.g. offsite drainage), so supplemental digital elevation data may be used to generate additional TIN file coverage that can be merged with the original TIN.

### 7.2.3.2 LiDAR Data

One supplemental source of digital elevation data available in North Carolina is the statewide Light Detection and Ranging (LiDAR) coverage that was developed for the NC Floodplain Mapping Program (FMP). The entire state has been mapped using LiDAR techniques to collect digital elevation data. These data and corresponding metadata are available for download, and can be accessed from FMP's website (http://www.ncfloodmaps.com).

### 7.2.3.3 USGS Digital Elevation Models and Local Government Topographic Data

Digital elevation model (DEM) data are available from the USGS National Elevation Dataset (NED). Procedures on how these data can be downloaded are provided on the National Map Viewer website (see 7.2.2). These DEMs may prove most useful for areas in bordering states; however, within the state, NC FMP's LiDAR coverage will likely be more current, higher resolution, and accurate than that available from the NED. Additionally, large municipalities and some counties have developed topographic and elevation data which may be publically available for use in drainage area determination.

### 7.2.4 Archived NCDOT Bridge and Culvert Survey and Hydraulic Design Reports

There are thousands of bridge and culvert design reports archived at the Hydraulics Unit (hardcopies and PDF electronic copies). They provide valuable hydrologic and hydraulic information, such as drainage area size, as well as discharge rates and associated computed water surface elevations, methods used for computations, flood history records, etc. Information provided on these reports are only as accurate as methods and technology available as of the date of the report. It is the design engineer's responsibility to verify the information on the report before relying on it.

### 7.2.5 FEMA Flood Insurance Studies

FEMA Flood Insurance Study (FIS) reports’ Summary of Discharges Tables are a good source for drainage areas and associated computed discharges for the FEMA hydraulic models. (See Section 7.4.1 for more information.)

### 7.3 Peak Discharge Design Frequency

Design frequency for NCDOT drainage structures is determined based on the roadway classification, traffic volume, level of service, flooding potential to properties, maintenance cost, etc. A summary of design frequencies that are typically used for NCDOT roadway drainage facilities is provided in Table 7-1. Consideration for site-specific conditions, such as upstream or downstream potential property impacts, existing level of service provided, length of time a temporary detour will be in place, etc. may warrant exceptions to these and should be discussed and agreed upon, preferably during the pre-design review.

|  | FREQUENCY (years) |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| ROADWAY <br> CLASSIFICATION | Bridges, <br> Culverts and <br> Cross Pipes | On Grade | At Sags <br> (without relief) | Ditches |
| Major Arterials (e.g. Interstates, <br> US, NC Routes) | 50 | 10 | 50 |  |
| Minor Arterials, Collectors, and <br> Local Roads | 25 | 10 | 25 | 10 |
| Temporary/Detours | 10 | - | - | 10 |

Table 7-1 Design Frequency

### 7.4 Peak Discharge Estimates

The design engineer should select from a number of acceptable peak discharge methods, depending upon the site's watershed characteristics. Table 7-2 lists peak discharge methods which are acceptable for NCDOT hydrologic studies. It also references the NCDOT Highway Hydrologic Charts (digitally corrected reproduction of the 1973 State Highway Commission Charts), which are applicable for limited use as discussed in Section 7.4.4 and Appendix C. It is the hydraulic engineer's responsibility to apply sound engineering judgment and to provide documented justification of methods used. Reported discharges should be expressed to two significant figures for 0.1 cfs to $10,000 \mathrm{cfs}$, and if higher, to three significant figures (examples: round 135.22 to 140 ; round 13,522 to 13,500 ), unless specifying discharges cited identically from a published FEMA Flood Insurance Study report.

| Feature | Hydrologic <br> Methods | FIS <br> (for NFIP <br> compliance) | USGS <br> Methods | Rational <br> Method <br> (up to 20 ac) | NCDOT Hwy. <br> Hydrologic <br> Charts |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Bridges | X | X |  |  | NRCS <br> Method <br> (for <br> routing) |
| Culverts | X | X |  |  | X |
| Storm Drain Systems |  |  | X | X | X |
| Cross Pipes ( $\leq 72$ in. dia.) | X | X | X | X | X |
| Gutter Spread |  |  | X |  |  |
| Ditches and Channels | X | X | X | X |  |
| BMP Devices |  |  | X |  | X |
| Natural Stream Design | X | X | X |  | X |
| Storage Facilities |  |  |  |  | X |
| Floodplain Impacts | X | X |  |  | X |

Table 7-2 Peak Discharge Method Selection

### 7.4.1 FEMA Flood Insurance Study

If a project study site is on a FEMA-regulated stream that is included in a published effective FEMA FIS, then the discharges specified in the FIS Summary of Discharges table should be used in the hydraulic model to demonstrate FEMA regulatory compliance. Those streams which were studied by detailed methods will typically list computed discharges for the 10-, $50-$, 100-, and 500-year recurrence intervals. Streams studied by limited detailed methods will only list the 100 -year discharge.

Copies of effective FIS reports can be viewed and downloaded online from NC Floodplain Mapping Program's (NC FMP) website (http://www.ncfloodmaps.com).

### 7.4.2 USGS Stream Gage Analysis

Precedence should be given to analysis of the published stream gage data records when a USGS gage exists at or near the study site. Published North Carolina flood frequency statistics from continuous record USGS gages are available from the Flood-Frequency Statistics USGS Gaged Sites web link
(http://nc.water.usgs.gov/flood/floodstats/gaged/index.html) on the NCDOT Hydraulics Unit website (https://connect.ncdot.gov/resources/hydro/pages/default.aspx).

### 7.4.2.1 Peak Discharge Estimation at Gaged Site

The above USGS website provides three types of statistical peak discharge estimates. The first is computed by fitting the recorded annual regulated peak flows to the log-Pearson Type III distribution using a localized computed sample skew. A second estimate that is provided is computed from the appropriate regionalized regression equation developed for the hydrologic area of the gage station location. The third, and presumably most accurate and reliable estimate provided combines the results of the first two into a weighted estimate for that gage station. Details on how these estimates are computed are discussed in USGS report SIR 2009-5158 (4). This report also discusses how flood-frequency peak discharge estimates at gaged sites can be adjusted (by transposition) to ungaged sites, as summarized in the following guidance.

### 7.4.2.2 Peak Discharge Estimation at Ungaged Site near Gaged Site

If the study site is not located at the location of a reference stream gage station on the same stream, and the drainage area at the study location is within fifty percent (50\%) of that of the reference gage station, it is acceptable to adjust (or transposition) the discharge from the gage station to compute discharge estimates at the study location. The recommended method for peak discharge transposition is detailed in USGS report SIR 2009-5158 (4). This method is not recommended if the difference in drainage areas between the two locations is greater than fifty percent (50\%). If the ungaged site is located between two gaged stations on the same stream, two peak discharge estimates can be made using the above procedure and hydrologic judgment applied to determine which is the more appropriate of the two.

### 7.4.2.3 Peak Discharge Estimation at Ungaged Site

In 2012, USGS launched the North Carolina StreamStats application website. In addition to the recommended use of this application for its automated drainage area delineation capabilities (see 7.2.1), this application is also recommended for use in computing discharges from USGS regression equations at ungaged sites. Rural discharge estimates are computed from the rural regional regression equations presented in SIR 2009-5158 (4). Urban and small rural basin discharge estimates are computed from the regression equations presented in reports SIR 2014-5030 (62), WRI 96-4084 (5), or USGS Fact Sheet 007-00 (63), as applicable. In the event that the StreamStats website is unavailable, refer to guidance in the referenced reports.

### 7.4.3 Rational Method

The Rational Method estimates the peak discharge (Q) in cubic feet per second (cfs) as a function of drainage area (A) in acres, mean rainfall intensity (I) in inches per hour (for a duration equal to the time of concentration, $\mathrm{t}_{\mathrm{c}}$ ), and a dimensionless runoff coefficient $(\mathrm{C})$. The Rational Formula is $\mathrm{Q}=\mathrm{CIA}$.

NRCS methods (49) for calculating $t_{c}$ should be used. Minimum value for $t_{c}$ should be 10 minutes. An upper limit of 20 acres drainage area is recommended for applicability of this method.

### 7.4.3.1 Rational Runoff Coefficient

The value of the runoff coefficient (C) increases with the imperviousness of the surface cover. Table 7-3 provides some commonly used values for various surface types (7). The higher values in the ranges shown should be used when the terrain slope is steep. Less permeable soils warrant higher range C values. Likewise, areas such as grassed medians and berms behind curb and gutter may also warrant higher C value because of reduced permeability due to soil compaction performed during construction.

| TYPE OF SURFACE | C |
| :---: | :---: |
| Pavement | $0.7-0.9$ |
| Gravel surfaces | $0.4-0.6$ |
| Industrial areas | $0.5-0.9$ |
| Residential (Single-family) | $0.3-0.5$ |
| Residential (Apartments, etc.) | $0.5-0.7$ |
| Grassed, steep slopes | $0.3-0.4$ |
| Grassed, flat slopes | $0.2-0.3$ |
| Woods / Forest | $0.1-0.2$ |

Table 7-3 Typical Rational Runoff Coefficients

### 7.4.3.2 Rainfall Intensity

Rainfall intensity (I) data can be obtained from the NOAA Atlas 14 published report (47) and corresponding Precipitation Frequency Data Server (PFDS) website, where "I" values are tabulated for a range of durations and storm event frequencies at user-selected locations. In the PFDS table, the duration which is closest to the computed time of concentration $\left(\mathrm{t}_{\mathrm{c}}\right)$ value will be used to obtain the corresponding "I" value to use in the Rational Formula. A minimum $\mathrm{t}_{\mathrm{c}}$ of ten (10) minutes should be used.

The website to access the PFDS is: http://hdsc.nws.noaa.gov/hdsc/pfds/pfds_map_cont.html See Appendix Q for an example of how to use the PFDS to find rainfall intensity values for a given project location.

Intensity values in GEOPAK Drainage (68) are hard coded into the Drainage Library and may not exactly match the NOAA Atlas 14 values for a given location, but should be relatively close. For routine storm drain system design, use the intensity values generated within GEOPAK Drainage.

### 7.4.4 NCDOT Highway Hydrologic Charts

The NCDOT Highway Hydrologic Charts, corrected and digitally reproduced from the 1973 State Highway Commission charts, are provided in Appendix C. They should primarily be used for sizing of small pipes.

### 7.4.5 NRCS Method - Storage Routing

Natural Resources Conservation Service (NRCS, formerly Soil Conservation Service) methods, presented in TR-55 (49) and TR-20 (48), are recommended for hydrographic storage routing. The TR-55 manual presents simplified hydrologic procedures for estimating flood hydrographs and peak discharges in small watersheds. The model begins with a rainfall uniformly imposed on the watershed over a specified time. Mass rainfall is then converted to mass runoff by using a runoff curve number (CN) which is based on soil type, land cover, impervious area, surface storage, infiltration rate, etc. Runoff is then converted to a hydrograph to develop peak discharges applying hydrograph routing procedures, runoff travel time, etc. TR-20 provides computer-aided hydrologic analyses for estimating flood hydrograph peak discharges in both small and large watersheds. For current soils data, the NRCS Web Soil Survey website is recommended (http://websoilsurvey.nrcs.usda.gov/app/HomePage.htm). Public domain software programs available from the Army Corps of Engineers Hydraulic Engineering Center (HEC) or NRCS are acceptable to perform hydrograph calculations and routing. Other hydrograph methods supported by FHWA and AASHTO $(1,2,7)$ may be used with approval of the State Hydraulics Engineer.

### 7.5 Accuracy of Hydrologic Estimates

The USGS scientists used various statistical methods to perform hydrologic analysis to develop regression equations for estimating peak discharges for both gaged and ungaged sites. It takes into account the complex geomorphic system of precipitation, evaporation, evapotranspiration, infiltration, overland flow, impoundments, channel flow, etc. The hydrologic analysis is not an exact science. The accuracy of the estimated discharges may vary significantly depending on location and other contributing factors. For example, the average standard error for the 10 -year peak discharge in the Piedmont region is $25 \%$; whereas, it is $73 \%$ for the 500 -year peak discharge in the Sand Hills region (62).

It can be argued that some hydrologic methods are more accurate than others; however, estimated discharges should be calibrated to locally observed or measured events. Methods should be applied within their limits of applicability and with understanding of the underlying assumptions and hydrologic principles supporting them. While detailed hydrologic analysis is not practicable and would be beyond the scope expected in normal NCDOT hydraulic engineering practice, the design engineer is encouraged to calibrate the results from any hydrologic procedure to historical data. For bridge hydraulic analysis (see Chapter 8), these NCDOT Guidelines recommend that comparison be made to at least one historical occurrence.

## Appendix E

Truck Transportation Data


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## Freight Demand and Destination Data

Freight demand and destination data was derived from three sources, detailed below.

1. STC activity centers - NCDOT guidelines for the STC contain categories of landmarks, destinations, and major hubs identified as "activity centers." These activity centers include destinations and land uses that will likely have higher demand for trucks than other destinations, including military bases, major airports, colleges/universities, and hospitals, among others. These activity centers were identified because of their dependence on the corridor for the shipping and receiving of goods via trucks, among other things.
2. Stakeholder organizations - The NCDOT project team developed a list of stakeholder organizations, including rail and freight representatives, economic development groups, and major employers with 500 or more employees. The stakeholder organizations that have a large facility were included as a freight destination.
3. Additional freight intensive land uses - Other land uses that would have higher demand for truck traffic, such as factories and distribution centers, were identified along the corridor through a desktop review of satellite imagery.

Freight destinations within 25 miles of the corridor were mapped (see Figures 3A-3C) and are listed in Table E-1.

Table E-1. Freight Demand and Destination Data - Corridor X

| Location Name | Type | Source* |
| :---: | :---: | :---: |
| Lenoir County |  |  |
| Electrolux Major Appliances | Manufacturing/Distribution Center | 2 |
| Global Transpark | Industrial/Business Park | 1 |
| Kinston-Regional Jetport | Airport | 2 |
| Moen Inc | Manufacturing/Distribution Center | 3 |
| Sanderson Farms, Inc. | Manufacturing/Distribution Center | 2 |
| Spirit Aero Systems Inc- Composite Fabrication Manufacturing Facility | Manufacturing/Distribution Center | 3 |
| UNC Lenoir Health Care | Hospital/Medical Center | 2 |
| West Pharmaceutical Services | Pharmaceutical Services | 3 |
| Onslow County |  |  |
| Albert J Ellis Airport | Airport | 1 |
| Camp Lejeune Range Control | Military Campus | 1, 2 |
| Coastal Carolina Community College | College/University | 2 |
| Jacksonville Mall | Shopping Center | 3 |
| Marine Corps Air Station, New River | Military Campus | 1, 2 |
| Martin Marietta - Onslow Quarry | Quarry | 3 |
| New River Air Station Chapel | Military Campus | 3 |
| Pitt County |  |  |
| Attends/Domar Healthcare WarehouseShipping and Receiving | Industrial/Business Park | 3 |
| DENSO Manufacturing North Carolina, IncGreenville Plant | Industrial/Business Park | 3 |
| Du Pont Sorona Kinston Pant | Factory | 3 |
| East Carolina University | College/University | 1, 2 |
| Grady-White Boats Inc | Manufacturing/Distribution Center | 3 |
| Greenville Mall | Shopping Center | 3 |
| Hyster-Yale Materials Handling Inc, | Industrial/Business Park | 2 |
| Hyster-Yale Group | Industrial/Business Park | 3 |
| Patheon Inc | Factory | 2 |
| Pitt Community College | College/University | 2 |
| Pitt-Greenville Airport | Airport | 1, 2 |
| Front Gate Shopping Center | Shopping Center | 3 |
| Thermo Fisher Scientific | Factory | 3 |
| Vidant Medical Center | Hospital/Medical Center | 1, 2 |
| Weyerhaeuser | Manufacturing/Distribution Center | 3 |

*Note: The source number corresponds to the following types of freight destinations:

1. STC Activity Centers
2. Stakeholder organizations
3. Additional freight intensive uses

## Truck Parking Data

Truck drivers are required to have a 30 -minute break every 8 hours and to stop driving after 14 consecutive hours due to federal hours of service (HOS) requirements. While helping to improve safety, these requirements often result in drivers searching for parking at predictable time intervals, typically at night. This puts a strain on key freight corridors that have insufficient truck parking relative to demand. When drivers can't find spaces at designated truck parking areas, they are faced with the following options:

- Parking in unauthorized and unsafe locations, such as abandoned parking lots or on freeway shoulders, that put personal safety of the driver at risk, or
- Continuing driving and run the risk of getting a citation for driving past the maximum allowable hours of service or driving while fatigued and getting into a harmful accident.
Table E-2 shows truck parking supply and availability along the corridor. Data was gathered as part of the North Carolina Truck Parking Study (January 2017). The table includes the name of the truck parking facility, the County, whether it is publicly or privately owned, and the number of spaces at the facility. For each facility, truck parking utilization is shown in Figures 3A-3C and in the table below. Truck parking facilities with "full utilization" are those that are fully occupied at least Monday through Friday.

Table E-2. Truck Parking Facility Data - Corridor X

| Location Name | Address | Facility Type | Number of Spaces | Utilization $^{1}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Pitt County |  |  |  |  |  |
| County Mart | 4787 NC-11, Bethel, <br> NC 27812 | Private | 38 | Available Spaces |  |
| Fuel Doc Travel Center | 2403 N Memorial Dr, <br> Greenville, NC 27834 | Private | 32 | Full Utilization |  |
| Lenoir County |  |  |  |  |  |
| Kangaroo Express | 559 Queen St, Grifton, <br> NC 28530 | Private | 20 | Full Utilization |  |

[^7]
## Truck Percentage Data

This appendix presents 2015 and 2019 truck percentage data for Corridor X of the North Carolina STC. Truck percentage data in Table E-3 is presented using the Annual Average Daily Traffic (AADT) GIS data the from NCDOT and is organized numerically by Route ID within each county. Route IDs correspond to individual segments of the roadway and are used by NCDOT to collect and organize traffic data; the Route IDs used for this report are based on the 2019 Route IDs and milepost segment limits. 2015 AADT and truck percentage data is included for the corresponding 2019 Route ID where it is available. The AADT data represent all vehicles counted for each Route ID, and the total truck percentages include both Single Unit trucks (FHWA Class 4-7) and Multi Unit Trucks (FHWA Class 8-13) (see Table E-4 for examples of each vehicle class). Truck data is only collected on segments of routes included in the National Highway System (NHS) and the North Carolina Truck Network. Truck percentage data on parallel corridors is included for locations where AADT data is not available on the STC corridor. Truck percentages (based on 2019 data) are shown in Figures 3A-3C.

Table E-3. Truck Percentage - 2019 and 2015 Annual Average Daily Traffic

| Route ID | Route | Beginning Milepost | End Milepost | 2015 <br> Annual <br> Average <br> Daily <br> Traffic <br> (AADT) | 2015 Total Truck Percentage | 2019 Annual Average Daily Traffic (AADT) | 2019 Total Truck Percentage | Change in Truck Percentage from 2015 to 2019 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Edgecombe County |  |  |  |  |  |  |  |  |
| 20000013033 | US-13 | 0 | 0.29 | 11,000 | 12.64\% | 650 | 11.75\% | -0.89\% |
| 20000013033 | US-13 | 0.29 | 0.55 | 10,000 | 12.31\% | 11,000 | 12.89\% | 0.58\% |
| Pitt County |  |  |  |  |  |  |  |  |
| 20000013074 | US-13 | 15.649 | 16.369 | 20,000 | 8.18\% | 22,000 | 7.78\% | -0.40\% |
| 20000013074 | US-13 | 16.369 | 17.209 | 16,000 | 10.38\% | 14,500 | 9.31\% | -1.07\% |
| 20000013074 | US-13 | 17.209 | 18.769 | 13,000 | 10.38\% | 14,000 | 9.31\% | -1.07\% |
| 20000013074 | US-13 | 18.769 | 21.229 | 14,000 | 10.38\% | 13,500 | 9.31\% | -1.07\% |
| 20000013074 | US-13 | 21.229 | 22.819 | 13,000 | 10.38\% | 13,500 | 9.31\% | -1.07\% |
| 20000013074 | US-13 | 22.819 | 24.639 | 15,000 | 10.38\% | 13,000 | 9.31\% | -1.07\% |
| 20000013074 | US-13 | 24.639 | 24.959 | 13,000 | 9.43\% | 14,000 | 10.78\% | 1.35\% |
| 20000013074 | US-13 | 24.959 | 26.204 | 12,000 | 11.59\% | 12,500 | 12.92\% | 1.33\% |
| 20000013074 | US-13 | 26.204 | 27.614 | 11,000 | 12.64\% | 650 | 11.75\% | -0.89\% |
| 20000264074 | US-264 | 13.253 | 14.774 | 13,000 | 11.11\% | 18,000 | 11.79\% | 0.68\% |
| 20000264074 | US-264 | 14.774 | 17.244 | 14,000 | 12.51\% | 20,000 | 14.34\% | 1.83\% |
| 20000264074 | US-13 | 17.244 | 20.104 | 12,000 | 13.07\% | 16,500 | 12.93\% | -0.14\% |
| 30000011074 | NC-11 | 0 | 1.92 | 13,000 | 5.65\% | 13,000 | 5.52\% | -0.13\% |
| 30000011074 | NC-11 | 1.92 | 2.17 | 17,000 | 5.65\% | 17,000 | 5.52\% | -0.13\% |
| 30000011074 | NC-11 | 2.17 | 3.66 | 19,000 | 5.65\% | 19,500 | 5.52\% | -0.13\% |
| 30000011074 | NC-11 | 3.66 | 4.97 | 20,000 | 5.65\% | 21,000 | 5.52\% | -0.13\% |
| 30000011074 | NC-11 | 4.97 | 6.81 | 18,000 | 5.65\% | 18,500 | 5.52\% | -0.13\% |
| 30000011074 | NC-11 | 6.81 | 7.99 | 18,000 | 6.36\% | 20,000 | 5.74\% | -0.62\% |
| 30000011074 | NC-11 | 7.99 | 9.76 | 18,000 | 6.36\% | 21,500 | 5.74\% | -0.62\% |
| 30000011074 | NC-11 | 9.76 | 10.77 | 20,000 | 6.36\% | 20,000 | 5.74\% | -0.62\% |
| 30000011074 | NC-11 | 10.77 | 11.29 | 22,000 | 5.54\% | 22,500 | 4.07\% | -1.47\% |
| 30000011074 | NC-11 | 11.29 | 11.83 | 25,000 | 5.54\% | 29,000 | 4.07\% | -1.47\% |
| 30000011074 | NC-11 | 11.83 | 12.18 | 36,000 | 5.54\% | 37,000 | 4.07\% | -1.47\% |
| 30000011074 | NC-11 | 12.18 | 13.07 | 32,000 | 5.54\% | 33,500 | 4.07\% | -1.47\% |
| 30000011074 | US-264 | 13.07 | 13.88 | 36,000 | 5.54\% | 36,500 | 4.07\% | -1.47\% |
| 30000011074 | US-264 | 13.88 | 14.652 | 27,000 | 5.82\% | 26,500 | 11.69\% | 5.87\% |
| 30000011074 | NC-11 | 14.652 | 15.31 | 25,000 | 5.82\% | 29,000 | 11.69\% | 5.87\% |
| 30000011074 | US-13 | 15.31 | 15.834 | 21,000 | 5.82\% | 20,500 | 11.69\% | 5.87\% |
| 40001467074 | US-264 | 0 | 0.5 | 21,000 | 4.17\% | 28,500 | 4.57\% | 0.40\% |
| 40001467074 | SR-1200 | 0.5 | 0.96 | 28,000 | 4.17\% | 30,500 | 4.57\% | 0.40\% |
| 40001467074 | SR-1203 | 0.96 | 1.494 | 31,000 | 4.17\% | 32,500 | 4.57\% | 0.40\% |
| 40001467074 | SR-1467 | 1.494 | 2.116 | 25,000 | 4.17\% | 25,500 | 4.57\% | 0.40\% |
| 50000904074 | US-13 | 2.154 | 2.561 | No data | No data | 10,000 | 0.00\% | N/A |
| Lenoir County |  |  |  |  |  |  |  |  |
| 20000070054 | US-70 | 9.682 | 11.35 | 22,000 | 10.09\% | 24,500 | 10.23\% | 0.14\% |
| 20000070054 | US-70 | 11.35 | 11.733 | 34,000 | 10.09\% | 31,000 | 10.23\% | 0.14\% |
| Lenoir County (cont.) |  |  |  |  |  |  |  |  |
| 20000070054 | US-70 | 12.226 | 13.431 | 27 | 8.43\% | 31,000 | 10.27\% | 1.84\% |


| Route ID | Route | Beginning Milepost | End Milepost | 2015 <br> Annual Average Daily Traffic (AADT) | 2015 Total Truck Percentage | 2019 <br> Annual <br> Average <br> Daily <br> Traffic <br> (AADT) | 2019 Total Truck Percentage | Change in Truck Percentage from 2015 to 2019 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 20000070054 | US-70 | 13.431 | 14.221 | 28,000 | 8.43\% | 30,500 | 10.27\% | 1.84\% |
| 20000070054 | US-70 | 14.221 | 15.417 | 20,000 | 11.63\% | 20,000 | 14.13\% | 2.50\% |
| 20000070054 | US-70 | 11.733 | 12.226 | 40,000 | 7.12\% | 40,500 | 7.01\% | -0.11\% |
| 20000258054 | US-258 | 0 | 0.456 | 4,300 | 10.77\% | 5,100 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 0.456 | 1.248 | 4,600 | 10.77\% | 5,200 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 1.248 | 2.158 | 5,100 | 10.77\% | 5,500 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 2.158 | 4.668 | 5,000 | 10.77\% | 5,100 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 4.668 | 8.562 | 5,400 | 10.77\% | 6,000 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 8.562 | 8.959 | 7,100 | 10.77\% | 7,200 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 8.959 | 10.982 | 6,900 | 10.77\% | 7,700 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 10.982 | 12.157 | 7,700 | 10.77\% | 8,300 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 12.157 | 12.348 | 9,200 | 10.77\% | 9,500 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 12.348 | 13.014 | 8,300 | 10.77\% | 7,800 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 13.014 | 13.675 | 8,300 | 10.77\% | 8,100 | 12.11\% | 1.34\% |
| 20000258054 | US-258 | 13.675 | 14.188 | 8,400 | 10.77\% | 8,300 | 12.11\% | 1.34\% |
| 20000258054 | US-70 | 14.188 | 14.568 | 9,400 | 10.77\% | 8,800 | 12.11\% | 1.34\% |
| 30000011054 | NC-11 | 23.568 | 25.124 | 14,000 | 7.76\% | 16,000 | 7.55\% | -0.21\% |
| 30000011054 | NC-11 | 25.124 | 26.584 | 15,000 | 7.76\% | 15,000 | 7.55\% | -0.21\% |
| 30000011054 | NC-11 | 26.584 | 27.794 | 16,000 | 7.76\% | 15,000 | 7.55\% | -0.21\% |
| 30000011054 | NC-11 | 27.794 | 28.557 | 16,000 | 7.76\% | 15,500 | 7.55\% | -0.21\% |
| 30000011054 | NC-11 | 28.557 | 28.914 | 12,000 | 7.76\% | 12,000 | 7.55\% | -0.21\% |
| 30000011054 | NC-11 | 28.914 | 29.684 | 13,000 | 5.65\% | 13,000 | 5.52\% | -0.13\% |
| 30000058054 | NC-58 | 13.272 | 13.602 | 5,600 | 5.41\% | 5,400 | 5.21\% | -0.20\% |
| 30000058054 | NC-58 | 13.602 | 14.653 | 4,200 | 5.41\% | 3,500 | 5.21\% | -0.20\% |
| 30000148054 | NC-58 | 0 | 1.581 | 2,000 | 3.95\% | 3,100 | 8.38\% | 4.43\% |
| 30000148054 | NC-148 | 1.581 | 2.244 | 4,500 | 3.95\% | 5,700 | 8.38\% | 4.43\% |
| 30000148054 | US-258 | 2.244 | 4.341 | 3,300 | 3.95\% | 5,100 | 8.38\% | 4.43\% |
| 30000148054 | US-70 | 4.341 | 8.092 | 3,000 | 15.01\% | 4,800 | 10.35\% | -4.66\% |
| 40001735054 | NC-11 | 1.77 | 2.109 | 1,500 | 0.00\% | 2,500 | 0.00\% | 0.00\% |
| 40001742054 | SR-1732 | 0 | 1.895 | 1,600 | 0.00\% | 1,500 | 0.00\% | 0.00\% |
| 40001742054 | NC-58 | 1.895 | 2.905 | 2,300 | 0.00\% | 2,200 | 0.00\% | 0.00\% |
| Jones County |  |  |  |  |  |  |  |  |
| 20000258052 | US-258 | 0 | 1.351 | 4,000 | 10.98\% | 5,000 | 10.72\% | -0.26\% |
| 20000258052 | US-258 | 1.351 | 3.28 | 4,600 | 10.77\% | 5,600 | 12.11\% | 1.34\% |
| 20000258052 | US-258 | 3.28 | 4.816 | 4,300 | 10.77\% | 5,100 | 12.11\% | 1.34\% |
| Onslow County |  |  |  |  |  |  |  |  |
| 20000258067 | US-258 | 0 | 0.92 | 19,000 | 4.98\% | 22,500 | 4.89\% | -0.09\% |
| 20000258067 | US-258 | 0.92 | 1.404 | 28,000 | 4.98\% | 30,000 | 4.89\% | -0.09\% |
| 20000258067 | US-258 | 1.404 | 1.881 | 38,000 | 7.89\% | 44,500 | 8.14\% | 0.25\% |
| 20000258067 | US-258 | 1.881 | 2.804 | 28,000 | 7.89\% | 37,000 | 8.14\% | 0.25\% |
| 20000258067 | US-258 | 2.804 | 3.566 | 27,000 | 7.89\% | 31,500 | 8.14\% | 0.25\% |
| 20000258067 | US-258 | 3.566 | 4.597 | 26,000 | 7.89\% | 29,000 | 8.14\% | 0.25\% |
| 20000258067 | US-258 | 4.597 | 6.424 | 25,000 | 7.89\% | 26,500 | 8.14\% | 0.25\% |


| Route ID | Route | Beginning Milepost | End Milepost | 2015 <br> Annual <br> Average Daily Traffic (AADT) | 2015 Total Truck Percentage | 2019 <br> Annual <br> Average <br> Daily <br> Traffic <br> (AADT) | 2019 Total Truck Percentage | Change in Truck Percentage from 2015 to 2019 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 20000258067 | US-258 | 6.424 | 8.46 | 17,000 | 8.21\% | 20,500 | 7.44\% | -0.77\% |
| 20000258067 | US-258 | 8.46 | 10.569 | 14,000 | 8.21\% | 17,000 | 7.44\% | -0.77\% |
| Onslow County (cont.) |  |  |  |  |  |  |  |  |
| 20000258067 | US-258 | 10.569 | 12.405 | 16,000 | 8.21\% | 19,500 | 7.44\% | -0.77\% |
| 20000258067 | US-258 | 12.405 | 13.028 | 14,000 | 8.21\% | 18,500 | 7.44\% | -0.77\% |
| 20000258067 | US-258 | 13.028 | 13.296 | 20,000 | 8.21\% | 24,000 | 7.44\% | -0.77\% |
| 20000258067 | US-258 | 13.296 | 14.07 | 17,000 | 8.21\% | 21,000 | 7.44\% | -0.77\% |
| 20000258067 | US-258 | 14.07 | 15.287 | 14,000 | 8.21\% | 16,500 | 7.44\% | -0.77\% |
| 20000258067 | US-258 | 15.287 | 17.009 | 4,300 | 10.98\% | 5,900 | 10.72\% | -0.26\% |
| 20000258067 | US-258 | 17.009 | 19.909 | 4,000 | 10.98\% | 5,700 | 10.72\% | -0.26\% |
| 20000258067 | US-258 | 19.909 | 20.968 | 4,000 | 10.98\% | 5,500 | 10.72\% | -0.26\% |
| 29000017067 | US-258 | 1.271 | 1.698 | 30,000 | 2.49\% | 34,000 | 2.81\% | 0.32\% |
| 29000017067 | US-17 | 1.698 | 2.382 | 26,000 | 2.49\% | 30,500 | 2.81\% | 0.32\% |
| 29000017067 | US-17 | 2.382 | 2.611 | 30,000 | 2.49\% | 30,500 | 2.81\% | 0.32\% |
| 39000024067 | US-17 | 2.757 | 3.327 | 4,400 | 2.45\% | 4,600 | 2.24\% | -0.21\% |
| 39000024067 | NC-24 | 3.327 | 3.427 | 12,000 | 2.45\% | 15,000 | 2.24\% | -0.21\% |
| 39000024067 | US-17 | 3.427 | 4.341 | 13,000 | 2.45\% | 14,500 | 2.24\% | -0.21\% |

Table E-4. Federal Highway Administration Vehicle Classification Definitions


Source: "FHWA Traffic Monitoring Guide. Appendix C: Vehicle Types" (2014)


Truck percentage data is shown on parallel corridors in locations where it is not available for the STC corridor because it is not open to traffic yet
${ }^{* *}$ The number shown within the circle refers to the total number of truck parking spaces at that parking facility
***Other category for Activity Centers includes colleges/universities, military camps, hospitals/medical centers, and airports

*Truck percentage data is shown on parallel corridors in locations where it is not available for the STC corridor because it is not open to traffic yet
**The number shown within the circle refers to the total number of truck parking spaces at that parking facility
***Other category for Activity Centers includes colleges/universities, military camps, hospitals/medical centers, and airports

*Truck percentage data is shown on parallel corridors in locations where it is not available for the STC corridor because it is not open to traffic yet
**The number shown within the circle refers to the total number of truck parking spaces at that parking facility
***Other category for Activity Centers includes colleges/universities, military camps, hospitals/medical centers, and airports

## Appendix F

Electric Charging Stations


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Figures 4A-4C. Electric Charging Stations


This map is accurate as of October 2021. For updated charging station locations and location addresses, please visit https://pluginnc.com/find-a-charging-station/


This map is accurate as of October 2021. For updated charging station locations and location addresses, please visit https://pluginnc.com/find-a-charging-station/


This map is accurate as of October 2021. For updated charging station locations and location addresses, please visit https://pluginnc.com/find-a-charging-station/

## Appendix G

Population and Employment Growth Data Statewide Model


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## Population and Employment Growth

This appendix presents base and future year population and employment growth for Corridor X of the North Carolina Strategic Transportation Corridors (STC). The following data is collected using the Traffic Analysis Zones (TAZ) of the North Carolina Statewide Model and is organized numerically by TAZ Number. TAZ within a 2-mile buffer area on both sides of the corridor were used to capture population and employment totals.

Table G-1. Population and Employment Growth - Statewide Model Traffic Analysis Zone

| TAZ Number | $\begin{gathered} \text { Population in } \\ 2015 \end{gathered}$ | Population in 2040 | Annual Population Growth (2015-2040) | Total Number of Employed Persons in 2015 | Total Number of Employed Persons in 2040 | Annual Employment Growth (2015-2040) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 890 | 114 | 112 | -0.1\% | 0 | 0 | 0.0\% |
| 1481 | 1,522 | 1,503 | -0.1\% | 258 | 288 | 0.4\% |
| 1482 | 2,729 | 2,711 | <0.0\% | 1,075 | 1,200 | 0.4\% |
| 1497 | 3,351 | 3,356 | <0.1\% | 453 | 481 | 0.2\% |
| 1498 | 11,546 | 11,482 | <0.0\% | 4,679 | 4,963 | 0.2\% |
| 1499 | 2,575 | 2,561 | <0.0\% | 1,941 | 2,059 | 0.2\% |
| 1500 | 11,085 | 11,000 | <0.0\% | 10,410 | 11,041 | 0.2\% |
| 1501 | 3,134 | 3,142 | <0.1\% | 1,943 | 2,061 | 0.2\% |
| 1502 | 2,547 | 2,549 | <0.1\% | 1,956 | 2,075 | 0.2\% |
| 1503 | 10,526 | 10,418 | <0.0\% | 6,110 | 6,480 | 0.2\% |
| 1505 | 2,789 | 2,755 | <0.0\% | 1,813 | 1,924 | 0.2\% |
| 1506 | 1,134 | 1,119 | -0.1\% | 157 | 167 | 0.2\% |
| 1507 | 2,493 | 2,460 | -0.1\% | 1,600 | 1,697 | 0.2\% |
| 1508 | 3,237 | 3,233 | <0.0\% | 951 | 1,009 | 0.2\% |
| 1875 | 5,585 | 9,323 | 2.1\% | 1,266 | 1,380 | 0.3\% |
| 1877 | 4,544 | 6,117 | 1.2\% | 288 | 307 | 0.3\% |
| 1878 | 14,637 | 23,399 | 1.9\% | 966 | 1,144 | 0.7\% |
| 1881 | 9,561 | 12,898 | 1.2\% | 977 | 1,026 | 0.2\% |
| 1882 | 2,964 | 3,996 | 1.2\% | 892 | 1,064 | 0.7\% |
| 1883 | 5,596 | 6,680 | 0.7\% | 4,945 | 5,164 | 0.2\% |
| 1885 | 11,110 | 13,996 | 0.9\% | 4,437 | 4,800 | 0.3\% |
| 1886 | 1,311 | 1,413 | 0.3\% | 6,934 | 7,542 | 0.3\% |
| 1889 | 1,643 | 2,217 | 1.2\% | 150 | 163 | 0.3\% |
| 1891 | 1,435 | 1,935 | 1.2\% | 33 | 64 | 2.7\% |
| 1892 | 1,389 | 1,872 | 1.2\% | 2,328 | 2,532 | 0.3\% |
| 1893 | 4,006 | 5,239 | 1.1\% | 935 | 987 | 0.2\% |
| 1894 | 3,767 | 4,435 | 0.7\% | 2,875 | 3,127 | 0.3\% |
| 1895 | 7,264 | 10,140 | 1.3\% | 1,474 | 1,603 | 0.3\% |
| 1896 | 20,350 | 26,386 | 1.0\% | 6,687 | 7,360 | 0.4\% |
| 1974 | 2,536 | 3,488 | 1.3\% | 307 | 563 | 2.5\% |
| 1975 | 2,607 | 3,631 | 1.3\% | 244 | 254 | 0.2\% |
| 1976 | 671 | 1,277 | 2.6\% | 26 | 28 | 0.3\% |
| 1977 | 279 | 405 | 1.5\% | 104 | 332 | 4.8\% |
| 1978 | 748 | 974 | 1.1\% | 200 | 209 | 0.2\% |

(Continued on next page)

Table F-2. Population and Employment Growth - Statewide Model Traffic Analysis Zone (Continued)

| TAZ Number | $\begin{array}{\|c} \text { Population in } \\ 2015 \end{array}$ | $\begin{array}{\|c} \text { Population in } \\ 2040 \end{array}$ | Annual Population Growth (2015-2040) | Total Number of Employed Persons in 2015 | Total Number of Employed Persons in 2040 | Annual Employment Growth (2015-2040) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1979 | 1,888 | 2,100 | 0.4\% | 367 | 381 | 0.1\% |
| 1980 | 1,221 | 1,751 | 1.5\% | 537 | 559 | 0.2\% |
| 1981 | 1,264 | 1,626 | 1.0\% | 4,836 | 6,383 | 1.1\% |
| 1982 | 399 | 570 | 1.4\% | 5,477 | 7,039 | 1.0\% |
| 1985 | 772 | 1,001 | 1.0\% | 307 | 321 | 0.2\% |
| 1986 | 1,540 | 1,731 | 0.5\% | 117 | 123 | 0.2\% |
| 1987 | 3,069 | 4,808 | 1.8\% | 923 | 1,265 | 1.3\% |
| 1988 | 1,673 | 2,767 | 2.0\% | 210 | 220 | 0.2\% |
| 1989 | 2,709 | 2,944 | 0.3\% | 1,403 | 1,818 | 1.0\% |
| 1990 | 3,616 | 5,558 | 1.7\% | 14,378 | 18,408 | 1.0\% |
| 1991 | 577 | 1,031 | 2.3\% | 701 | 731 | 0.2\% |
| 1993 | 2,525 | 3,621 | 1.5\% | 3,062 | 4,052 | 1.1\% |
| 2001 | 5,566 | 7,883 | 1.4\% | 1,436 | 2,076 | 1.5\% |
| 2011 | 9,922 | 18,940 | 2.6\% | 3,194 | 4,755 | 1.6\% |
| 2012 | 427 | 661 | 1.8\% | 110 | 115 | 0.2\% |
| 2013 | 1,638 | 2,215 | 1.2\% | 157 | 165 | 0.2\% |
| 2014 | 762 | 1,484 | 2.7\% | 62 | 282 | 6.2\% |
| 2015 | 413 | 689 | 2.1\% | 79 | 84 | 0.2\% |
| 2019 | 1,095 | 1,532 | 1.4\% | 576 | 864 | 1.6\% |
| 2020 | 2,800 | 4,195 | 1.6\% | 945 | 986 | 0.2\% |
| 2025 | 1,216 | 1,716 | 1.4\% | 360 | 1,069 | 4.4\% |
| 2026 | 694 | 1,355 | 2.7\% | 702 | 1,477 | 3.0\% |
| 2028 | 2,721 | 3,540 | 1.1\% | 765 | 1,284 | 2.1\% |
| 2029 | 1,031 | 1,289 | 0.9\% | 573 | 598 | 0.2\% |
| 2031 | 1,754 | 1,795 | 0.1\% | 488 | 2,707 | 7.1\% |
| 2032 | 1,868 | 2,969 | 1.9\% | 463 | 2,568 | 7.1\% |
| 2034 | 1,250 | 1,384 | 0.4\% | 609 | 3,380 | 7.1\% |
| 2042 | 159 | 216 | 1.2\% | 18 | 104 | 7.3\% |
| 2043 | 4,242 | 6,215 | 1.5\% | 1,094 | 6,064 | 7.1\% |



TAZ Number on the map correlates to TAZ Number in Appendix F of the Mobility Analysis Report. Additional population and employment data can be found in the data tables.
*TAZ Number is the 1-4 digit solid, bold number in map.


TAZ Number on the map correlates to TAZ Number in Appendix F of the Mobility Analysis Report. Additional population and employment data can be found in the data tables.
*TAZ Number is the 1-4 digit solid, bold number in map.


TAZ Number on the map correlates to TAZ Number in Appendix F of the Mobility Analysis Report. Additional population and employment data can be found in the data tables.
*TAZ Number is the 1-4 digit solid, bold number in map.


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*TAZ Number is the 1-4 digit solid, bold number in map.


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*TAZ Number is the 1-4 digit solid, bold number in map.


TAZ Number on the map correlates to TAZ Number in Appendix F of the Mobility Analysis Report. Additional population and employment data can be found in the data tables.
*TAZ Number is the 1-4 digit solid, bold number in map.

## Appendix H

## Population and Employment Growth Data Regional Models



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## Population and Employment Growth

This appendix presents base and future year population and employment growth for Corridor X of the North Carolina Strategic Transportation Corridors (STC). The following data is collected using the Kinston, Jacksonville, and Greenville Regional Traffic Model Traffic Analysis Zones (TAZ) and is organized numerically by TAZ Number. TAZs within a 2-mile buffer area on both sides of the corridor were used to capture population and employment totals.

Table H-1. Population and Employment Growth - Kinston Regional Model Traffic Analysis Zone

| TAZ Number | Population in 2010 | $\begin{array}{\|c} \text { Population in } \\ 2040 \end{array}$ | Annual Population Growth (2010-2040) | Total Number of Employed Persons in 2010 | Total Number of Employed Persons in 2040 | Annual Employment Growth (2010-2040) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 74 | 111 | 111 | 0.0\% | 9 | 11 | 0.7\% |
| 85 | 4 | 4 | 0.0\% | 2 | 2 | 0.0\% |
| 86 | 374 | 374 | 0.0\% | 30 | 32 | 0.2\% |
| 87 | 4 | 4 | 0.0\% | 4 | 5 | 0.7\% |
| 102 | 57 | 57 | 0.0\% | 108 | 129 | 0.6\% |
| 103 | 8 | 8 | 0.0\% | 830 | 942 | 0.4\% |
| 104 | 55 | 55 | 0.0\% | 7 | 9 | 0.8\% |
| 105 | 104 | 104 | 0.0\% | 787 | 833 | 0.2\% |
| 106 | 280 | 280 | 0.0\% | 340 | 369 | 0.3\% |
| 107 | 195 | 195 | 0.0\% | 412 | 529 | 0.8\% |
| 108 | 102 | 102 | 0.0\% | 128 | 144 | 0.4\% |
| 109 | 340 | 340 | 0.0\% | 38 | 53 | 1.1\% |
| 110 | 231 | 231 | 0.0\% | 55 | 75 | 1.0\% |
| 111 | 1554 | 1554 | 0.0\% | 3514 | 3791 | 0.3\% |
| 112 | 61 | 61 | 0.0\% | 64 | 75 | 0.5\% |
| 113 | 282 | 282 | 0.0\% | 5 | 5 | 0.0\% |
| 114 | 705 | 705 | 0.0\% | 25 | 25 | 0.0\% |
| 115 | 1808 | 1808 | 0.0\% | 81 | 94 | 0.5\% |
| 116 | 87 | 87 | 0.0\% | 90 | 106 | 0.5\% |
| 117 | 5 | 5 | 0.0\% | 9 | 9 | 0.0\% |
| 118 | 485 | 485 | 0.0\% | 11 | 12 | 0.3\% |
| 119 | 215 | 215 | 0.0\% | 10 | 11 | 0.3\% |
| 120 | 150 | 150 | 0.0\% | 27 | 29 | 0.2\% |
| 121 | 264 | 264 | 0.0\% | 249 | 290 | 0.5\% |
| 122 | 217 | 217 | 0.0\% | 276 | 458 | 1.7\% |
| 123 | 1251 | 1251 | 0.0\% | 45 | 104 | 2.8\% |
| 124 | 0 | 0 | 0.0\% | 84 | 104 | 0.7\% |
| 125 | 52 | 52 | 0.0\% | 1 | 151 | 18.2\% |
| 127 | 95 | 95 | 0.0\% | 1 | 1 | 0.0\% |
| 130 | 131 | 131 | 0.0\% | 5 | 27 | 5.8\% |
| 131 | 54 | 54 | 0.0\% | 5 | 19 | 4.6\% |
| 132 | 27 | 27 | 0.0\% | 335 | 606 | 2.0\% |
| 133 | 378 | 378 | 0.0\% | 13 | 615 | 13.7\% |
| 134 | 18 | 18 | 0.0\% | 1086 | 1410 | 0.9\% |

(Continued on next page)

Table H-1. Population and Employment Growth - Kinston Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | Population in 2010 | Population in 2040 | Annual Population Growth (2010-2040) | Total Number of Employed Persons in 2010 | Total Number of Employed Persons in 2040 | Annual Employment Growth (2010-2040) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 135 | 146 | 146 | 0.0\% | 43 | 312 | 6.8\% |
| 136 | 0 | 0 | 0.0\% | 30 | 86 | 3.6\% |
| 137 | 1072 | 1072 | 0.0\% | 630 | 750 | 0.6\% |
| 138 | 399 | 399 | 0.0\% | 16 | 16 | 0.0\% |
| 139 | 191 | 191 | 0.0\% | 172 | 182 | 0.2\% |
| 144 | 596 | 596 | 0.0\% | 249 | 260 | 0.1\% |
| 148 | 351 | 351 | 0.0\% | 281 | 293 | 0.1\% |
| 161 | 482 | 482 | 0.0\% | 320 | 340 | 0.2\% |
| 162 | 306 | 306 | 0.0\% | 155 | 158 | 0.1\% |
| 171 | 2 | 2 | 0.0\% | 373 | 438 | 0.5\% |
| 172 | 269 | 269 | 0.0\% | 509 | 615 | 0.6\% |
| 173 | 30 | 30 | 0.0\% | 55 | 57 | 0.1\% |
| 189 | 18 | 18 | 0.0\% | 305 | 322 | 0.2\% |
| 193 | 120 | 120 | 0.0\% | 9 | 10 | 0.4\% |
| 194 | 963 | 963 | 0.0\% | 602 | 636 | 0.2\% |
| 195 | 72 | 72 | 0.0\% | 219 | 240 | 0.3\% |
| 205 | 364 | 364 | 0.0\% | 32 | 34 | 0.2\% |
| 207 | 90 | 90 | 0.0\% | 0 | 0 | 0.0\% |
| 210 | 48 | 48 | 0.0\% | 132 | 42 | -3.7\% |
| 211 | 33 | 33 | 0.0\% | 14 | 252 | 10.1\% |
| 212 | 111 | 111 | 0.0\% | 1 | 15 | 9.4\% |
| 213 | 53 | 53 | 0.0\% | 56 | 0 | -100.0\% |
| 214 | 20 | 20 | 0.0\% | 1 | 67 | 15.0\% |
| 215 | 0 | 0 | 0.0\% | 12 | 0 | -100.0\% |
| 216 | 325 | 325 | 0.0\% | 3 | 13 | 5.0\% |
| 217 | 66 | 66 | 0.0\% | 34 | 4 | -6.9\% |
| 218 | 22 | 22 | 0.0\% | 71 | 53 | -1.0\% |
| 219 | 30 | 30 | 0.0\% | 7 | 83 | 8.6\% |
| 220 | 49 | 49 | 0.0\% | 0 | 0 | 0.0\% |
| 221 | 66 | 66 | 0.0\% | 97 | 12 | -6.7\% |
| 222 | 210 | 210 | 0.0\% | 2 | 97 | 13.8\% |
| 223 | 9 | 9 | 0.0\% | 37 | 2 | -9.3\% |
| 224 | 30 | 30 | 0.0\% | 1 | 39 | 13.0\% |
| 225 | 63 | 63 | 0.0\% | 2 | 0 | -100.0\% |

(Continued on next page)

Table H-1. Population and Employment Growth - Kinston Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | $\begin{array}{\|c} \text { Population in } \\ 2010 \end{array}$ | $\begin{array}{\|c} \text { Population in } \\ 2040 \end{array}$ | Annual Population Growth (2010-2040) | Total Number of Employed Persons in 2010 | Total Number of Employed Persons in 2040 | Annual Employment Growth (2010-2040) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 226 | 0 | 0 | 0.0\% | 4 | 2 | -2.3\% |
| 228 | 3 | 3 | 0.0\% | 30 | 0 | -100.0\% |
| 229 | 10 | 10 | 0.0\% | 1 | 40 | 13.1\% |
| 237 | 129 | 129 | 0.0\% | 1 | 4 | 4.7\% |
| 238 | 11 | 11 | 0.0\% | 10 | 0 | -100.0\% |
| 239 | 76 | 76 | 0.0\% | 3 | 11 | 4.4\% |
| 240 | 114 | 114 | 0.0\% | 1 | 4 | 4.7\% |
| 241 | 283 | 283 | 0.0\% | 3 | 0 | -100.0\% |
| 246 | 68 | 68 | 0.0\% | 1 | 2 | 2.3\% |
| 247 | 85 | 85 | 0.0\% | 0 | 0 | 0.0\% |
| 252 | 147 | 147 | 0.0\% | 27 | 0 | -100.0\% |
| 287 | 245 | 245 | 0.0\% | 8 | 9 | 0.4\% |
| 288 | 140 | 140 | 0.0\% | 1 | 9 | 7.6\% |
| 292 | 425 | 425 | 0.0\% | 21 | 636 | 12.0\% |
| 293 | 80 | 80 | 0.0\% | 40 | 21 | -2.1\% |
| 296 | 209 | 209 | 0.0\% | 23 | 98 | 5.0\% |
| 297 | 62 | 62 | 0.0\% | 7 | 23 | 4.0\% |
| 298 | 3 | 3 | 0.0\% | 2 | 7 | 4.3\% |
| 299 | 0 | 0 | 0.0\% | 64 | 2 | -10.9\% |
| 300 | 48 | 48 | 0.0\% | 45 | 70 | 1.5\% |
| 301 | 34 | 34 | 0.0\% | 0 | 0 | 0.0\% |
| 302 | 650 | 650 | 0.0\% | 5 | 5 | 0.0\% |
| 303 | 68 | 68 | 0.0\% | 206 | 237 | 0.5\% |
| 304 | 175 | 175 | 0.0\% | 213 | 231 | 0.3\% |
| 305 | 119 | 119 | 0.0\% | 249 | 261 | 0.2\% |
| 306 | 22 | 22 | 0.0\% | 39 | 39 | 0.0\% |
| 307 | 1028 | 1028 | 0.0\% | 6 | 6 | 0.0\% |
| 308 | 174 | 174 | 0.0\% | 25 | 31 | 0.7\% |
| 309 | 274 | 274 | 0.0\% | 0 | 0 | 0.0\% |
| 310 | 52 | 52 | 0.0\% | 42 | 45 | 0.2\% |
| 313 | 136 | 136 | 0.0\% | 1 | 1 | 0.0\% |
| 314 | 209 | 209 | 0.0\% | 4 | 5 | 0.7\% |
| 315 | 183 | 183 | 0.0\% | 54 | 56 | 0.1\% |
| 317 | 357 | 357 | 0.0\% | 0 | 0 | 0.0\% |
| 321 | 114 | 114 | 0.0\% | 5 | 5 | 0.0\% |

(Continued on next page)
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Table H-1. Population and Employment Growth - Kinston Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | Population in <br> 2010 | Population in <br> $\mathbf{2 0 4 0}$ | Annual <br> Population <br> Growth <br> $(2010-2040)$ | Total Number <br> of Employed <br> Persons in <br> 2010 | Total Number <br> of Employed <br> Persons in <br> 2040 | Annual <br> Employment <br> Growth <br> $(2010-2040)$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 325 | 750 | 750 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 331 | 0 | 0 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 332 | 56 | 56 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 333 | 67 | 67 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 334 | 98 | 98 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 335 | 185 | 185 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 336 | 0 | 0 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 337 | 0 | 0 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 338 | 67 | 67 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 339 | 13 | 13 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 340 | 0 | 0 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 341 | 203 | 203 | $0.0 \%$ | 10 | 7 | $-1.2 \%$ |
| 342 | 134 | 134 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 343 | 0 | 0 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 344 | 471 | 471 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 345 | 0 | 0 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 346 | 0 | 0 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 347 | 50 | 50 | $0.0 \%$ | 86 | 96 | $0.4 \%$ |
| 348 | 30 | 30 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 349 | 11 | 11 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |

Table H-2. Population and Employment Growth - Jacksonville Regional Model Traffic Analysis Zone

| TAZ Number | $\begin{array}{\|c} \text { Population in } \\ 2010 \end{array}$ | Population in 2040 | Annual Population Growth (2010-2040) | Total Number of Employed Persons in 2010 | Total Number of Employed Persons in 2040 | Annual Employment Growth (2010-2040) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 39 | 215 | 356 | 1.7\% | 10 | 17 | 1.8\% |
| 40 | 621 | 1029 | 1.7\% | 45 | 79 | 1.9\% |
| 41 | 347 | 575 | 1.7\% | 10 | 17 | 1.8\% |
| 42 | 295 | 489 | 1.7\% | 13 | 23 | 1.9\% |
| 43 | 359 | 595 | 1.7\% | 16 | 28 | 1.9\% |
| 44 | 70 | 116 | 1.7\% | 4 | 7 | 1.9\% |
| 45 | 27 | 44 | 1.6\% | 32 | 56 | 1.9\% |
| 48 | 636 | 1986 | 3.9\% | 4 | 7 | 1.9\% |
| 49 | 72 | 224 | 3.9\% | 21 | 37 | 1.9\% |
| 50 | 536 | 1674 | 3.9\% | 6 | 10 | 1.7\% |
| 53 | 209 | 652 | 3.9\% | 0 | 0 | 0.0\% |
| 54 | 200 | 624 | 3.9\% | 31 | 54 | 1.9\% |
| 55 | 266 | 784 | 3.7\% | 28 | 49 | 1.9\% |
| 56 | 93 | 154 | 1.7\% | 8 | 14 | 1.9\% |
| 57 | 77 | 127 | 1.7\% | 46 | 81 | 1.9\% |
| 58 | 314 | 520 | 1.7\% | 75 | 132 | 1.9\% |
| 59 | 164 | 271 | 1.7\% | 228 | 403 | 1.9\% |
| 60 | 299 | 535 | 2.0\% | 52 | 92 | 1.9\% |
| 61 | 28 | 46 | 1.7\% | 12 | 21 | 1.9\% |
| 62 | 171 | 283 | 1.7\% | 208 | 368 | 1.9\% |
| 63 | 203 | 336 | 1.7\% | 4 | 7 | 1.9\% |
| 64 | 22 | 36 | 1.7\% | 311 | 550 | 1.9\% |
| 65 | 367 | 608 | 1.7\% | 1 | 1 | 0.0\% |
| 66 | 104 | 172 | 1.7\% | 23 | 40 | 1.9\% |
| 67 | 128 | 212 | 1.7\% | 2 | 3 | 1.4\% |
| 68 | 254 | 421 | 1.7\% | 9 | 15 | 1.7\% |
| 76 | 185 | 306 | 1.7\% | 2 | 3 | 1.4\% |
| 77 | 64 | 106 | 1.7\% | 1 | 1 | 0.0\% |
| 79 | 1125 | 3514 | 3.9\% | 57 | 100 | 1.9\% |
| 80 | 1164 | 1930 | 1.7\% | 157 | 277 | 1.9\% |
| 81 | 978 | 1621 | 1.7\% | 14 | 24 | 1.8\% |
| 82 | 690 | 1144 | 1.7\% | 192 | 339 | 1.9\% |
| 83 | 296 | 490 | 1.7\% | 140 | 247 | 1.9\% |
| 84 | 1136 | 1883 | 1.7\% | 62 | 109 | 1.9\% |
| 85 | 655 | 1086 | 1.7\% | 308 | 545 | 1.9\% |

Table H-2. Population and Employment Growth - Jacksonville Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | $\begin{array}{\|c} \text { Population in } \\ 2010 \end{array}$ | $\begin{array}{\|c} \text { Population in } \\ 2040 \end{array}$ | Annual Population Growth (2010-2040) | Total Number of Employed Persons in 2010 | Total Number of Employed Persons in 2040 | Annual Employment Growth (2010-2040) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 86 | 293 | 485 | 1.7\% | 66 | 116 | 1.9\% |
| 87 | 3438 | 5700 | 1.7\% | 122 | 215 | 1.9\% |
| 103 | 845 | 1401 | 1.7\% | 11 | 19 | 1.8\% |
| 158 | 944 | 1565 | 1.7\% | 6 | 10 | 1.7\% |
| 159 | 52 | 86 | 1.7\% | 101 | 178 | 1.9\% |
| 160 | 310 | 514 | 1.7\% | 594 | 1051 | 1.9\% |
| 161 | 838 | 1389 | 1.7\% | 10 | 17 | 1.8\% |
| 162 | 500 | 829 | 1.7\% | 27 | 47 | 1.9\% |
| 163 | 823 | 1364 | 1.7\% | 15 | 26 | 1.9\% |
| 164 | 1118 | 1853 | 1.7\% | 115 | 203 | 1.9\% |
| 165 | 235 | 389 | 1.7\% | 851 | 1506 | 1.9\% |
| 166 | 565 | 936 | 1.7\% | 806 | 1426 | 1.9\% |
| 167 | 191 | 191 | 0.0\% | 3 | 5 | 1.7\% |
| 168 | 11 | 11 | 0.0\% | 251 | 444 | 1.9\% |
| 169 | 312 | 312 | 0.0\% | 26 | 46 | 1.9\% |
| 170 | 1389 | 2303 | 1.7\% | 337 | 596 | 1.9\% |
| 171 | 1143 | 1895 | 1.7\% | 95 | 168 | 1.9\% |
| 172 | 687 | 1139 | 1.7\% | 107 | 189 | 1.9\% |
| 179 | 1458 | 2417 | 1.7\% | 89 | 157 | 1.9\% |
| 186 | 2703 | 2703 | 0.0\% | 2801 | 4958 | 1.9\% |
| 187 | 5188 | 7415 | 1.2\% | 507 | 897 | 1.9\% |
| 188 | 1190 | 1973 | 1.7\% | 616 | 1090 | 1.9\% |
| 189 | 1200 | 1989 | 1.7\% | 21 | 37 | 1.9\% |
| 190 | 1895 | 2316 | 0.7\% | 997 | 1765 | 1.9\% |
| 191 | 679 | 1125 | 1.7\% | 551 | 975 | 1.9\% |
| 192 | 86 | 142 | 1.7\% | 43 | 76 | 1.9\% |
| 193 | 2 | 3 | 1.4\% | 53 | 93 | 1.9\% |
| 194 | 17 | 28 | 1.7\% | 142 | 251 | 1.9\% |
| 195 | 304 | 504 | 1.7\% | 138 | 244 | 1.9\% |
| 196 | 16 | 26 | 1.6\% | 94 | 166 | 1.9\% |
| 197 | 1 | 1 | 0.0\% | 125 | 221 | 1.9\% |
| 198 | 0 | 0 | 0.0\% | 608 | 1076 | 1.9\% |
| 199 | 0 | 0 | 0.0\% | 144 | 254 | 1.9\% |
| 200 | 8 | 13 | 1.6\% | 74 | 131 | 1.9\% |
| 201 | 3 | 4 | 1.0\% | 35 | 61 | 1.9\% |

Table H-2. Population and Employment Growth - Jacksonville Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | Population in 2010 | Population in 2040 | Annual Population Growth (2010-2040) | Total Number of Employed Persons in 2010 | Total Number of Employed Persons in 2040 | Annual Employment Growth (2010-2040) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 202 | 120 | 198 | 1.7\% | 551 | 975 | 1.9\% |
| 203 | 590 | 978 | 1.7\% | 331 | 585 | 1.9\% |
| 204 | 143 | 237 | 1.7\% | 182 | 322 | 1.9\% |
| 205 | 353 | 585 | 1.7\% | 219 | 387 | 1.9\% |
| 206 | 1433 | 2376 | 1.7\% | 1836 | 3250 | 1.9\% |
| 214 | 0 | 0 | 0.0\% | 1131 | 2002 | 1.9\% |
| 216 | 1505 | 2495 | 1.7\% | 151 | 267 | 1.9\% |
| 218 | 2284 | 2284 | 0.0\% | 66 | 116 | 1.9\% |
| 219 | 2435 | 2435 | 0.0\% | 526 | 931 | 1.9\% |
| 220 | 1670 | 1670 | 0.0\% | 976 | 1727 | 1.9\% |
| 221 | 0 | 0 | 0.0\% | 105 | 185 | 1.9\% |
| 239 | 774 | 774 | 0.0\% | 349 | 617 | 1.9\% |
| 240 | 739 | 739 | 0.0\% | 357 | 632 | 1.9\% |
| 241 | 306 | 306 | 0.0\% | 106 | 187 | 1.9\% |
| 242 | 151 | 151 | 0.0\% | 294 | 520 | 1.9\% |
| 243 | 551 | 913 | 1.7\% | 542 | 959 | 1.9\% |
| 244 | 1 | 1 | 0.0\% | 91 | 161 | 1.9\% |
| 245 | 592 | 981 | 1.7\% | 20 | 35 | 1.9\% |
| 246 | 71 | 71 | 0.0\% | 650 | 1150 | 1.9\% |
| 247 | 219 | 219 | 0.0\% | 44 | 77 | 1.9\% |
| 248 | 104 | 104 | 0.0\% | 35 | 61 | 1.9\% |
| 249 | 343 | 343 | 0.0\% | 2423 | 4289 | 1.9\% |
| 250 | 273 | 273 | 0.0\% | 5 | 8 | 1.6\% |
| 251 | 15 | 15 | 0.0\% | 383 | 678 | 1.9\% |
| 252 | 1 | 1 | 0.0\% | 1308 | 2315 | 1.9\% |
| 253 | 61 | 61 | 0.0\% | 855 | 1513 | 1.9\% |
| 254 | 0 | 0 | 0.0\% | 427 | 755 | 1.9\% |
| 501 | 4742 | 5508 | 0.5\% | 257 | 454 | 1.9\% |

Table H-3. Population and Employment Growth - Greenville Regional Model Traffic Analysis Zone

| TAZ Number | $\begin{array}{\|c} \text { Population in } \\ 2016 \end{array}$ | $\begin{array}{\|l} \text { Population in } \\ 2045 \end{array}$ | Annual Population Growth (2016-2045) | Total Number of Employed Persons in 2016 | Total Number of Employed Persons in 2045 | Annual Employment Growth (2016-2045) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 72 | 1017 | 1100 | 0.3\% | 65 | 70 | 0.3\% |
| 73 | 21 | 23 | 0.3\% | 215 | 233 | 0.3\% |
| 74 | 0 | 0 | 0.0\% | 8430 | 9126 | 0.3\% |
| 76 | 0 | 0 | 0.0\% | 381 | 413 | 0.3\% |
| 78 | 0 | 0 | 0.0\% | 1104 | 1196 | 0.3\% |
| 79 | 619 | 670 | 0.3\% | 393 | 426 | 0.3\% |
| 80 | 1074 | 1162 | 0.3\% | 69 | 75 | 0.3\% |
| 81 | 140 | 151 | 0.3\% | 449 | 486 | 0.3\% |
| 82 | 1075 | 1163 | 0.3\% | 45 | 48 | 0.2\% |
| 84 | 107 | 116 | 0.3\% | 195 | 210 | 0.3\% |
| 85 | 3523 | 3811 | 0.3\% | 273 | 295 | 0.3\% |
| 87 | 862 | 933 | 0.3\% | 264 | 286 | 0.3\% |
| 88 | 1170 | 1266 | 0.3\% | 40 | 43 | 0.2\% |
| 89 | 141 | 153 | 0.3\% | 56 | 61 | 0.3\% |
| 90 | 3146 | 3403 | 0.3\% | 669 | 726 | 0.3\% |
| 92 | 1488 | 1610 | 0.3\% | 499 | 540 | 0.3\% |
| 115 | 30 | 32 | 0.2\% | 150 | 163 | 0.3\% |
| 116 | 120 | 130 | 0.3\% | 5 | 5 | 0.0\% |
| 117 | 22 | 24 | 0.3\% | 190 | 206 | 0.3\% |
| 118 | 132 | 143 | 0.3\% | 206 | 224 | 0.3\% |
| 119 | 28 | 30 | 0.2\% | 15 | 16 | 0.2\% |
| 121 | 0 | 0 | 0.0\% | 442 | 478 | 0.3\% |
| 122 | 0 | 0 | 0.0\% | 673 | 731 | 0.3\% |
| 123 | 28 | 30 | 0.2\% | 1244 | 1347 | 0.3\% |
| 124 | 70 | 76 | 0.3\% | 214 | 233 | 0.3\% |
| 128 | 201 | 217 | 0.3\% | 59 | 64 | 0.3\% |
| 130 | 0 | 0 | 0.0\% | 0 | 0 | 0.0\% |
| 131 | 0 | 0 | 0.0\% | 207 | 225 | 0.3\% |
| 132 | 923 | 999 | 0.3\% | 50 | 53 | 0.2\% |
| 159 | 50 | 54 | 0.3\% | 32 | 35 | 0.3\% |
| 160 | 5 | 5 | 0.0\% | 679 | 736 | 0.3\% |
| 161 | 15 | 16 | 0.2\% | 1344 | 1456 | 0.3\% |
| 168 | 0 | 0 | 0.0\% | 2397 | 2597 | 0.3\% |
| 196 | 1104 | 1194 | 0.3\% | 1522 | 1649 | 0.3\% |
| 197 | 362 | 392 | 0.3\% | 85 | 92 | 0.3\% |
| 198 | 1841 | 1992 | 0.3\% | 93 | 100 | 0.3\% |

Table H-3. Population and Employment Growth - Greenville Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | $\begin{array}{\|c} \text { Population in } \\ 2016 \end{array}$ | Population in 2045 | Annual Population Growth (2016-2045) | Total Number of Employed Persons in 2016 | Total Number of Employed Persons in 2045 | Annual Employment Growth (2016-2045) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 200 | 1781 | 1927 | 0.3\% | 600 | 651 | 0.3\% |
| 215 | 1288 | 1393 | 0.3\% | 239 | 258 | 0.3\% |
| 217 | 990 | 1071 | 0.3\% | 44 | 48 | 0.3\% |
| 222 | 55 | 60 | 0.3\% | 1 | 1 | 0.0\% |
| 224 | 119 | 129 | 0.3\% | 1 | 1 | 0.0\% |
| 263 | 42 | 45 | 0.2\% | 3 | 3 | 0.0\% |
| 264 | 428 | 463 | 0.3\% | 10 | 10 | 0.0\% |
| 265 | 20 | 22 | 0.3\% | 0 | 0 | 0.0\% |
| 266 | 374 | 405 | 0.3\% | 5 | 5 | 0.0\% |
| 267 | 53 | 57 | 0.3\% | 10 | 10 | 0.0\% |
| 268 | 266 | 288 | 0.3\% | 41 | 44 | 0.2\% |
| 269 | 271 | 293 | 0.3\% | 17 | 18 | 0.2\% |
| 270 | 98 | 106 | 0.3\% | 13 | 14 | 0.3\% |
| 271 | 80 | 87 | 0.3\% | 15 | 16 | 0.2\% |
| 272 | 158 | 171 | 0.3\% | 20 | 21 | 0.2\% |
| 273 | 88 | 95 | 0.3\% | 6 | 6 | 0.0\% |
| 315 | 368 | 398 | 0.3\% | 5 | 5 | 0.0\% |
| 316 | 427 | 462 | 0.3\% | 286 | 310 | 0.3\% |
| 318 | 281 | 304 | 0.3\% | 76 | 82 | 0.3\% |
| 320 | 189 | 204 | 0.3\% | 137 | 148 | 0.3\% |
| 322 | 6 | 6 | 0.0\% | 6 | 6 | 0.0\% |
| 323 | 593 | 642 | 0.3\% | 41 | 44 | 0.2\% |
| 324 | 32 | 35 | 0.3\% | 0 | 0 | 0.0\% |
| 325 | 87 | 94 | 0.3\% | 48 | 51 | 0.2\% |
| 335 | 10 | 11 | 0.3\% | 0 | 0 | 0.0\% |
| 336 | 116 | 125 | 0.3\% | 277 | 302 | 0.3\% |
| 341 | 850 | 920 | 0.3\% | 43 | 46 | 0.2\% |
| 342 | 698 | 755 | 0.3\% | 35 | 38 | 0.3\% |
| 343 | 302 | 327 | 0.3\% | 316 | 342 | 0.3\% |
| 344 | 131 | 142 | 0.3\% | 3 | 3 | 0.0\% |
| 345 | 22 | 24 | 0.3\% | 3 | 3 | 0.0\% |
| 346 | 16 | 17 | 0.2\% | 15 | 16 | 0.2\% |
| 347 | 224 | 242 | 0.3\% | 15 | 15 | 0.0\% |
| 348 | 520 | 563 | 0.3\% | 30 | 32 | 0.2\% |
| 349 | 188 | 203 | 0.3\% | 57 | 61 | 0.2\% |

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Kimley-Horn Appendix H - Corridor X | March 2022

Table H-3. Population and Employment Growth - Greenville Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | $\begin{array}{\|c} \text { Population in } \\ 2016 \end{array}$ | Population in 2045 | Annual Population Growth (2016-2045) | Total Number of Employed Persons in 2016 | Total Number of Employed Persons in 2045 | Annual Employment Growth (2016-2045) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 350 | 107 | 116 | 0.3\% | 58 | 63 | 0.3\% |
| 351 | 77 | 83 | 0.3\% | 4 | 4 | 0.0\% |
| 353 | 273 | 295 | 0.3\% | 10 | 10 | 0.0\% |
| 355 | 8 | 9 | 0.4\% | 0 | 0 | 0.0\% |
| 356 | 50 | 54 | 0.3\% | 8 | 8 | 0.0\% |
| 357 | 480 | 519 | 0.3\% | 42 | 46 | 0.3\% |
| 358 | 98 | 106 | 0.3\% | 10 | 11 | 0.3\% |
| 359 | 804 | 870 | 0.3\% | 15 | 16 | 0.2\% |
| 363 | 471 | 510 | 0.3\% | 79 | 85 | 0.3\% |
| 364 | 96 | 104 | 0.3\% | 37 | 39 | 0.2\% |
| 365 | 540 | 584 | 0.3\% | 299 | 325 | 0.3\% |
| 366 | 418 | 452 | 0.3\% | 114 | 124 | 0.3\% |
| 367 | 51 | 55 | 0.3\% | 2 | 2 | 0.0\% |
| 368 | 0 | 0 | 0.0\% | 62 | 67 | 0.3\% |
| 370 | 469 | 507 | 0.3\% | 207 | 224 | 0.3\% |
| 371 | 147 | 159 | 0.3\% | 37 | 40 | 0.3\% |
| 372 | 19 | 21 | 0.3\% | 9 | 9 | 0.0\% |
| 373 | 3 | 3 | 0.0\% | 0 | 0 | 0.0\% |
| 374 | 507 | 548 | 0.3\% | 48 | 51 | 0.2\% |
| 376 | 1166 | 1261 | 0.3\% | 92 | 99 | 0.3\% |
| 377 | 1062 | 1149 | 0.3\% | 37 | 41 | 0.4\% |
| 381 | 1219 | 1319 | 0.3\% | 43 | 46 | 0.2\% |
| 383 | 61 | 66 | 0.3\% | 10 | 10 | 0.0\% |
| 390 | 120 | 130 | 0.3\% | 18 | 20 | 0.4\% |
| 391 | 289 | 313 | 0.3\% | 32 | 35 | 0.3\% |
| 401 | 62 | 67 | 0.3\% | 8 | 9 | 0.4\% |
| 407 | 16 | 17 | 0.2\% | 0 | 0 | 0.0\% |
| 408 | 62 | 67 | 0.3\% | 19 | 20 | 0.2\% |
| 409 | 30 | 32 | 0.2\% | 0 | 0 | 0.0\% |
| 410 | 39 | 42 | 0.3\% | 3 | 3 | 0.0\% |
| 411 | 10 | 11 | 0.3\% | 0 | 0 | 0.0\% |
| 413 | 74 | 80 | 0.3\% | 26 | 28 | 0.3\% |
| 414 | 61 | 66 | 0.3\% | 15 | 16 | 0.2\% |
| 417 | 6 | 6 | 0.0\% | 0 | 0 | 0.0\% |
| 418 | 103 | 111 | 0.3\% | 10 | 11 | 0.3\% |

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Table H-3. Population and Employment Growth - Greenville Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | $\begin{array}{\|c} \text { Population in } \\ 2016 \end{array}$ | $\begin{array}{\|c} \text { Population in } \\ 2045 \end{array}$ | Annual Population Growth (2016-2045) | Total Number of Employed Persons in 2016 | Total Number of Employed Persons in 2045 | Annual Employment Growth (2016-2045) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 419 | 24 | 26 | 0.3\% | 0 | 0 | 0.0\% |
| 420 | 138 | 149 | 0.3\% | 12 | 13 | 0.3\% |
| 472 | 23 | 25 | 0.3\% | 4 | 4 | 0.0\% |
| 473 | 553 | 598 | 0.3\% | 265 | 285 | 0.3\% |
| 474 | 197 | 213 | 0.3\% | 272 | 294 | 0.3\% |
| 475 | 197 | 213 | 0.3\% | 250 | 272 | 0.3\% |
| 476 | 50 | 54 | 0.3\% | 5 | 5 | 0.0\% |
| 477 | 67 | 72 | 0.2\% | 5 | 5 | 0.0\% |
| 478 | 429 | 464 | 0.3\% | 369 | 399 | 0.3\% |
| 479 | 0 | 0 | 0.0\% | 250 | 271 | 0.3\% |
| 480 | 229 | 248 | 0.3\% | 222 | 240 | 0.3\% |
| 481 | 826 | 894 | 0.3\% | 253 | 274 | 0.3\% |
| 482 | 279 | 302 | 0.3\% | 53 | 56 | 0.2\% |
| 483 | 61 | 66 | 0.3\% | 3 | 3 | 0.0\% |
| 484 | 58 | 63 | 0.3\% | 0 | 0 | 0.0\% |
| 485 | 678 | 733 | 0.3\% | 121 | 131 | 0.3\% |
| 486 | 110 | 119 | 0.3\% | 125 | 135 | 0.3\% |
| 487 | 85 | 92 | 0.3\% | 451 | 489 | 0.3\% |
| 488 | 94 | 102 | 0.3\% | 51 | 54 | 0.2\% |
| 489 | 206 | 223 | 0.3\% | 19 | 20 | 0.2\% |
| 490 | 182 | 197 | 0.3\% | 8 | 8 | 0.0\% |
| 491 | 41 | 44 | 0.2\% | 4 | 4 | 0.0\% |
| 494 | 325 | 352 | 0.3\% | 35 | 38 | 0.3\% |
| 495 | 78 | 84 | 0.3\% | 11 | 12 | 0.3\% |
| 496 | 29 | 31 | 0.2\% | 17 | 18 | 0.2\% |
| 497 | 26 | 28 | 0.3\% | 0 | 0 | 0.0\% |
| 498 | 19 | 21 | 0.3\% | 4 | 4 | 0.0\% |
| 499 | 143 | 155 | 0.3\% | 12 | 13 | 0.3\% |
| 501 | 390 | 422 | 0.3\% | 10 | 10 | 0.0\% |
| 503 | 235 | 254 | 0.3\% | 57 | 62 | 0.3\% |
| 504 | 91 | 98 | 0.3\% | 10 | 11 | 0.3\% |
| 505 | 91 | 98 | 0.3\% | 17 | 18 | 0.2\% |
| 506 | 293 | 317 | 0.3\% | 27 | 29 | 0.2\% |
| 509 | 273 | 295 | 0.3\% | 105 | 114 | 0.3\% |
| 510 | 183 | 198 | 0.3\% | 48 | 52 | 0.3\% |

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Kimley-Horn Appendix H - Corridor X | March 2022

Table H-3. Population and Employment Growth - Greenville Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | $\begin{array}{\|c} \text { Population in } \\ 2016 \end{array}$ | $\begin{array}{\|c} \text { Population in } \\ 2045 \end{array}$ | Annual Population Growth (2016-2045) | Total Number of Employed Persons in 2016 | Total Number of Employed Persons in 2045 | Annual Employment Growth (2016-2045) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 511 | 297 | 321 | 0.3\% | 262 | 283 | 0.3\% |
| 514 | 34 | 37 | 0.3\% | 0 | 0 | 0.0\% |
| 515 | 187 | 202 | 0.3\% | 11 | 12 | 0.3\% |
| 516 | 321 | 347 | 0.3\% | 5 | 5 | 0.0\% |
| 517 | 341 | 369 | 0.3\% | 25 | 27 | 0.3\% |
| 518 | 13 | 14 | 0.3\% | 3 | 3 | 0.0\% |
| 519 | 44 | 48 | 0.3\% | 3 | 3 | 0.0\% |
| 520 | 224 | 242 | 0.3\% | 16 | 17 | 0.2\% |
| 521 | 144 | 156 | 0.3\% | 3 | 3 | 0.0\% |
| 522 | 114 | 123 | 0.3\% | 0 | 0 | 0.0\% |
| 523 | 86 | 93 | 0.3\% | 200 | 216 | 0.3\% |
| 524 | 229 | 248 | 0.3\% | 10 | 11 | 0.3\% |
| 525 | 277 | 300 | 0.3\% | 8 | 8 | 0.0\% |
| 526 | 25 | 27 | 0.3\% | 63 | 68 | 0.3\% |
| 527 | 26 | 28 | 0.3\% | 17 | 18 | 0.2\% |
| 536 | 88 | 95 | 0.3\% | 10 | 11 | 0.3\% |
| 537 | 118 | 128 | 0.3\% | 42 | 46 | 0.3\% |
| 538 | 179 | 194 | 0.3\% | 15 | 15 | 0.0\% |
| 539 | 124 | 134 | 0.3\% | 31 | 33 | 0.2\% |
| 540 | 49 | 53 | 0.3\% | 10 | 10 | 0.0\% |
| 541 | 50 | 54 | 0.3\% | 0 | 0 | 0.0\% |
| 545 | 54 | 58 | 0.2\% | 17 | 19 | 0.4\% |
| 546 | 32 | 35 | 0.3\% | 0 | 0 | 0.0\% |
| 620 | 57 | 62 | 0.3\% | 15 | 17 | 0.4\% |
| 621 | 65 | 70 | 0.3\% | 132 | 142 | 0.3\% |
| 622 | 108 | 117 | 0.3\% | 6 | 6 | 0.0\% |
| 624 | 18 | 19 | 0.2\% | 9 | 10 | 0.4\% |
| 625 | 33 | 36 | 0.3\% | 5 | 5 | 0.0\% |
| 626 | 0 | 0 | 0.0\% | 0 | 0 | 0.0\% |
| 627 | 3 | 3 | 0.0\% | 4 | 4 | 0.0\% |
| 628 | 71 | 77 | 0.3\% | 13 | 13 | 0.0\% |
| 629 | 86 | 93 | 0.3\% | 15 | 16 | 0.2\% |
| 630 | 68 | 74 | 0.3\% | 5 | 5 | 0.0\% |
| 631 | 240 | 260 | 0.3\% | 50 | 54 | 0.3\% |
| 632 | 131 | 142 | 0.3\% | 55 | 60 | 0.3\% |

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Kimley-Horn Appendix H - Corridor X | March 2022

Table H-3. Population and Employment Growth - Greenville Regional Model Traffic Analysis Zone (Continued)

| TAZ Number | Population in <br> $\mathbf{2 0 1 6}$ | Population in <br> $\mathbf{2 0 4 5}$ | Annual <br> Population <br> Growth <br> $(2016-2045)$ | Total Number <br> of Employed <br> Persons in <br> 2016 | Total Number <br> of Employed <br> Persons in <br> $\mathbf{2 0 4 5}$ | Annual <br> Employment <br> Growth <br> $(2016-2045)$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 633 | 38 | 41 | $0.3 \%$ | 2 | 2 | $0.0 \%$ |
| 634 | 339 | 367 | $0.3 \%$ | 0 | 0 | $0.0 \%$ |
| 635 | 6 | 6 | $0.0 \%$ | 0 | 0 | $0.0 \%$ |
| 636 | 9 | 10 | $0.4 \%$ | 0 | 0 | $0.0 \%$ |
| 637 | 120 | 130 | $0.3 \%$ | 0 | 0 | $0.0 \%$ |
| 638 | 31 | 34 | $0.3 \%$ | 0 | 0 | $0.0 \%$ |
| 640 | 142 | 154 | $0.3 \%$ | 67 | 72 | $0.2 \%$ |
| 641 | 275 | 298 | $0.3 \%$ | 6 | 6 | $0.0 \%$ |
| 642 | 163 | 176 | $0.3 \%$ | 7 | 7 | $0.0 \%$ |
| 643 | 41 | 44 | $0.2 \%$ | 0 | 0 | $0.0 \%$ |
| 644 | 33 | 36 | $0.3 \%$ | 0 | 0 | $0.0 \%$ |
| 645 | 133 | 144 | $0.3 \%$ | 20 | 21 | $0.2 \%$ |
| 646 | 14 | 15 | $0.2 \%$ | 0 | 0 | $0.0 \%$ |
| 647 | 21 | 23 | $0.3 \%$ | 0 | 0 | $0.0 \%$ |
| 650 | 27 | 29 | $0.2 \%$ | 5 | 5 | $0.0 \%$ |
| 652 | 43 | 47 | $0.3 \%$ | 149 | 162 | $0.3 \%$ |


 *TAZ Number is the 1-4 digit solid, bold number in map. Kinston, and Jacksonville Annual Growth is 2010-2040, the Greenville Annual Growth is 2016-2045.

 *TAZ Number is the 1-4 digit solid, bold number in map. Kinston, and Jacksonville Annual Growth is 2010-2040, the Greenville Annual Growth is 2016-2045.

 *TAZ Number is the 1-4 digit solid, bold number in map. Kinston, and Jacksonville Annual Growth is 2010-2040, the Greenville Annual Growth is 2016-2045.

 *TAZ Number is the 1-4 digit solid, bold number in map. Kinston, and Jacksonville Annual Growth is 2010-2040, the Greenville Annual Growth is 2016-2045.

 *TAZ Number is the 1-4 digit solid, bold number in map. Kinston, and Jacksonville Annual Growth is 2010-2040, the Greenville Annual Growth is 2016-2045.


*TAZ Number is the 1-4 digit solid, bold number in map. Kinston, and Jacksonville Annual Growth is 2010-2040, the Greenville Annual Growth is 2016-2045.


[^0]:    *AADT = Annual Average Daily Traffic

[^1]:    Note: Shaded grey fields indicate a change from 2018 Conditions to 2040 scenarios

[^2]:    ${ }^{1}$ North Carolina Statewide Multimodal Freight Plan, Freight Flow Tool Reference Guide: https://connect.ncdot.gov/projects/planning/Statewide-Freight-Plan/Documents/Freight_Tool_User_Guide.pdf

[^3]:    2 "Other SC" refers to the remainder of South Carolina not including the Greenville and Charleston metros. "Other VA" refers to the remainder of Virginia not including the Washington, DC, Virginia Beach, and Richmond metros.

[^4]:    3 "Other SC" refers to the remainder of South Carolina not including the Greenville and Charleston metros.

[^5]:    ${ }^{4}$ Class Density Ratio: The crash density of the study area versus the average crash density of similar facilities.
    Severity Index: A measure of the severity of a crash or series of crashes Locations with a high severity index have higher than average injury rates and/or more severe injuries.
    Critical Crash Rate Ratio: The actual crash rate for the study area versus the critical crash rate.

[^6]:    *Road inundation incidents were collected on existing roadways for the portion of the corridor that is not yet existing or under construction.
    ${ }^{* *}$ Gray represents each segment's design frequency as defined in Table 7-1 of the NCDOT Guidelines for Drainage Studies and Hydraulic Design, included
    in Appendix D.
    ***"N/A" indicates that there are no road inundation incidents in a given recurrence interval in the segment.

[^7]:    ${ }^{1}$ Based on 2017 Truck Parking Study

